

TOTAL WEIGHT = 3 X 130 = 391 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY 2100F 1.8E	SPF
C - F	2x4	DRY 2100F 1.8E	SPF
F - H	2x4	DRY 1650F 1.5E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
N - B	2x6	DRY 1650F 1.5E	SPF
N - K	2x4	DRY 1650F 1.5E	SPF
K - I	2x4	DRY 1650F 1.5E	SPF
ALL WEBS	2x4	DRY 1650F 1.5E	SPF
EXCEPT			
M - D	2x3	DRY No.2	SPF
L - E	2x3	DRY No.2	SPF
J - F	2x3	DRY No.2	SPF
J - G	2x3	DRY No.2	SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	2621	0	2621	0	5-8	3-0
I	2353	0	2353	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				SOIL
	SNOW	LIVE	PERMLIVE	WIND	DEAD		
N	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J. DBS = 16-0-0 . CBF = 187 LBS.
 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I. DBS = 12-0-0 . CBF = 191 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)			FACTORED VERT. LOAD (PLF)			WEBS MAX. FACTORED FORCE (LBS)		
	FR-TO	FROM	TO	LC1	MAX	UNBRAC	MEMB.	FORCE	MAX
A-B	0 / 42	-139.4	-139.4	0.23	(1)	10.00	M-D	-259 / 74	0.05 (1)
B-C	-4716 / 0	-139.4	-139.4	0.78	(1)	3.31	D-L	-1150 / 0	0.71 (1)
C-D	-4716 / 0	-139.4	-139.4	0.78	(1)	3.31	L-E	0 / 565	0.13 (1)
D-E	-3630 / 0	-139.4	-139.4	0.58	(1)	3.97	E-J	-1866 / 0	0.51 (1)
E-F	-2100 / 0	-139.4	-139.4	0.55	(1)	4.90	J-F	0 / 683	0.15 (1)
F-G	-2093 / 0	-139.4	-139.4	0.32	(1)	4.93	J-G	0 / 766	0.17 (1)
G-H	0 / 24	-139.4	-139.4	0.39	(1)	10.00	G-I	-2546 / 0	0.50 (1)
I-H	-240 / 0	0.0	0.0	0.07	(1)	7.81	B-M	0 / 4517	0.53 (1)
N-B	-2565 / 0	0.0	0.0	0.10	(1)	7.23			
N-M	0 / 0	-17.5	-17.5	0.17	(4)	10.00			
M-L	0 / 4501	-17.5	-17.5	0.59	(1)	10.00			
L-K	0 / 3445	-17.5	-17.5	0.55	(1)	10.00			
K-J	0 / 3445	-17.5	-17.5	0.55	(1)	10.00			
J-I	0 / 1511	-17.5	-17.5	0.32	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/967 (0.37")

CSI: TC=0.78/1.00 (B-D:1), BC=0.59/1.00 (L-M:1), WB=0.71/1.00 (D-L:1), SSI=0.43/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00
 AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
 JSI METAL= 0.86 (K) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.25	3.25
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-p	MT20	4.0	4.0		
G	TMVW-t	MT20	3.5	6.0	1.50	2.75
H	TMV+p	MT20	1.5	4.0		
I	BMVW-t	MT20	4.0	6.0	1.50	3.00
J	BMVW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.5	6.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.75
M	BMVW-t	MT20	5.0	8.0	2.50	2.50



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200656	C1	3	1	Wood Creek Const.	2 of 75

C1X

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:20:36 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-GnJHeu5e74XQcLjul?LGsn2OrhCqLZHqS9oUxZzWhrf

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
N	BMV1+p	MT20	2.0	6.0	Edge 1.00

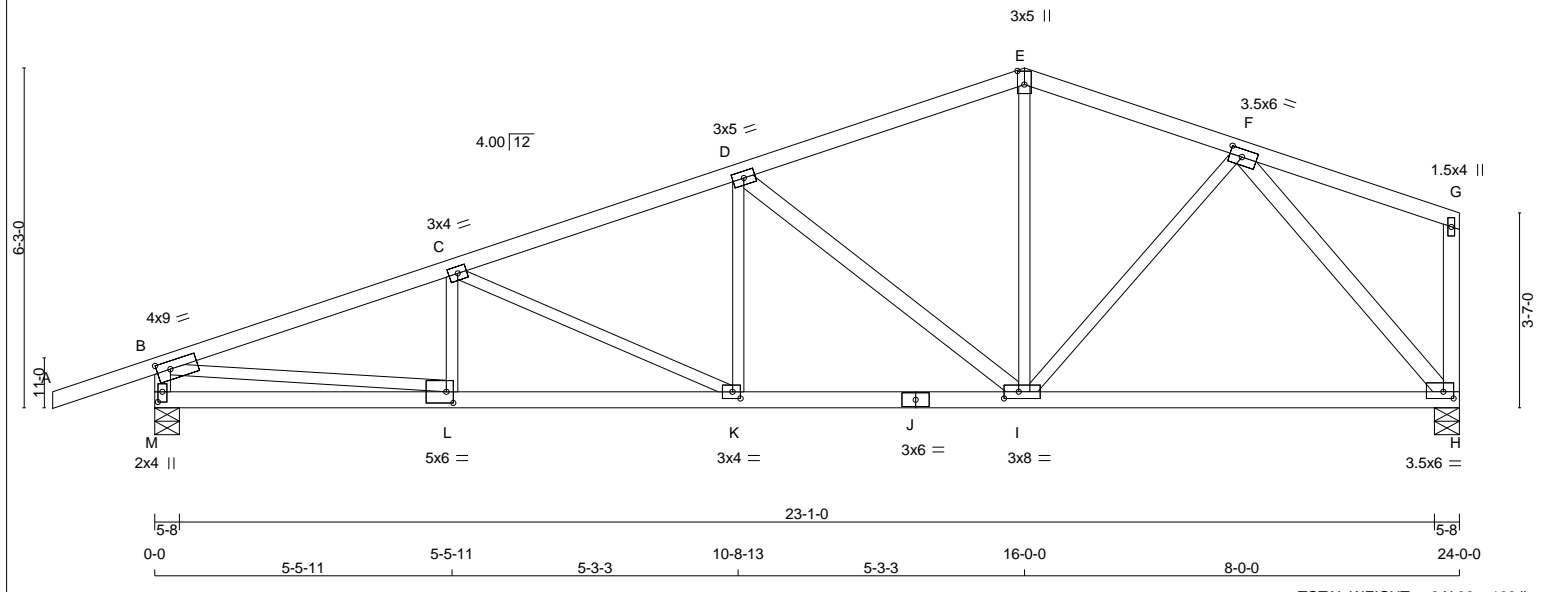
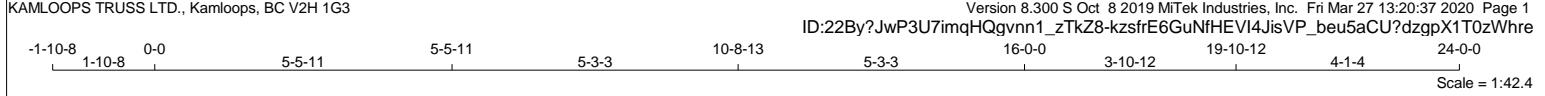
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY 1650F 1.5E	SPF
E - G	2x4	DRY 1650F 1.5E	SPF
M - B	2x4	DRY 1650F 1.5E	SPF
H - G	2x4	DRY 1650F 1.5E	SPF
M - J	2x4	DRY 1650F 1.5E	SPF
J - H	2x4	DRY 1650F 1.5E	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
D - I	2x4	DRY 1650F 1.5E	SPF
F - H	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	5.0		
E	TTW+p	MT20	3.0	5.0	3.00	1.50
F	TMVW-t	MT20	3.5	6.0	1.75	2.75
G	TMV+p	MT20	1.5	4.0		
H	BMVW1-t	MT20	3.5	6.0	1.50	2.25
I	BMVWV-t	MT20	3.0	8.0	1.50	3.25
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	3.0	4.0	1.50	1.75
L	BMVW-t	MT20	5.0	6.0	2.50	1.50
M	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2150	0	2150	0	5-8	3-8
H	1883	0	1883	0	5-8	2-1

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1485	1178 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0
H	1303	1015 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S		W E B S					
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)				
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00	L-C	-294 / 38	0.05 (1)
B-C	-3565 / 0	-139.4	-139.4	0.50 (1)	3.78	C-K	-711 / 0	0.41 (1)
C-D	-2904 / 0	-139.4	-139.4	0.39 (1)	4.26	K-D	0 / 380	0.09 (1)
D-E	-1818 / 0	-139.4	-139.4	0.38 (1)	5.11	D-I	-1346 / 0	0.60 (1)
E-F	-1816 / 0	-139.4	-139.4	0.25 (1)	5.29	I-E	0 / 627	0.14 (1)
F-G	0 / 21	-139.4	-139.4	0.30 (1)	10.00	I-F	0 / 470	0.11 (1)
M-B	-2102 / 0	0.0	0.0	0.13 (1)	6.58	B-L	0 / 3423	0.77 (1)
H-G	-214 / 0	0.0	0.0	0.03 (1)	7.81	F-H	-2132 / 0	0.71 (1)
M-L	0 / 0	-17.5	-17.5	0.09 (4)	10.00			
L-K	0 / 3401	-17.5	-17.5	0.46 (1)	10.00			
K-J	0 / 2756	-17.5	-17.5	0.43 (1)	10.00			
J-I	0 / 2756	-17.5	-17.5	0.43 (1)	10.00			
I-H	0 / 1396	-17.5	-17.5	0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.50/1.00 (B-C:1), BC=0.46/1.00 (K-L:1), WB=0.77/1.00 (B-L:1), SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

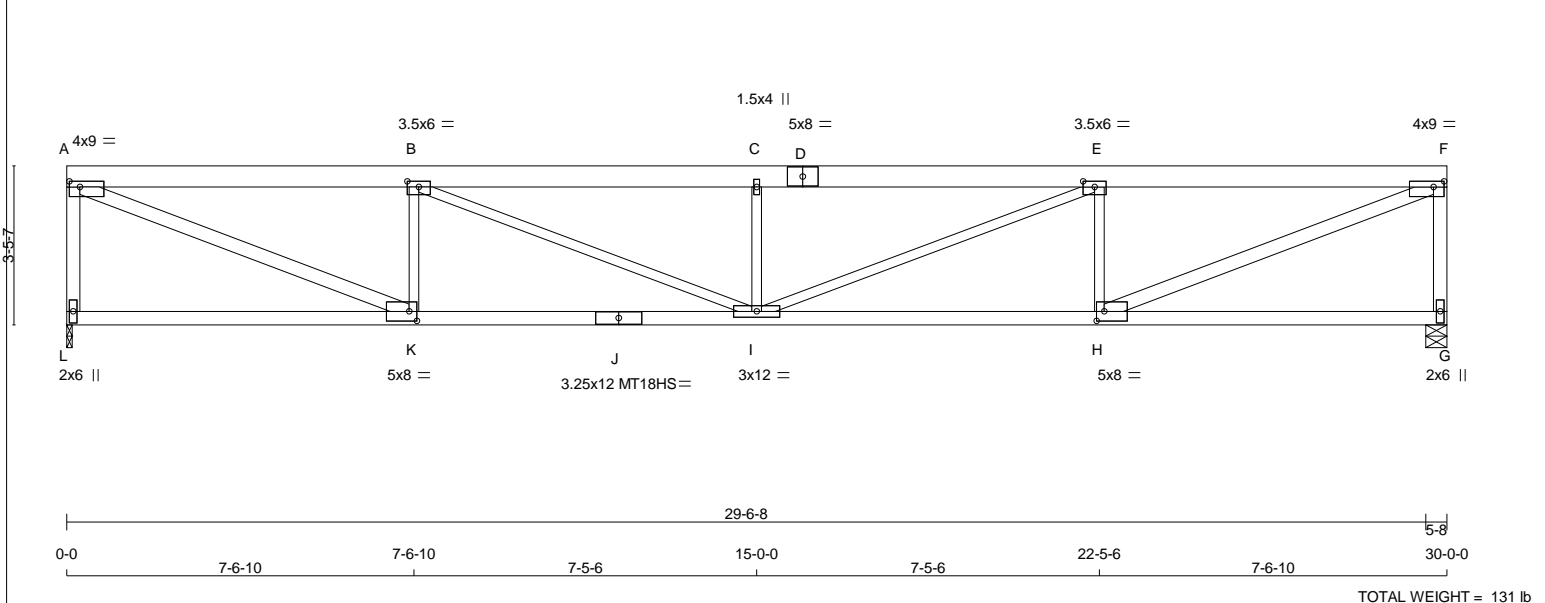
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.80 (J) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.



TOTAL WEIGHT = 131 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY 1650F 1.5E	SPF
A - D	2x6	DRY 1650F 1.5E	SPF
D - F	2x6	DRY 1650F 1.5E	SPF
G - F	2x4	DRY 1650F 1.5E	SPF
L - J	2x4	DRY 1650F 1.5E	SPF
J - G	2x4	DRY 1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
A - K	2x4	DRY 1650F 1.5E	SPF
H - F	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	1.50	2.75
B	TMVW-t	MT20	3.5	6.0	1.50	3.00
C	TMW+w	MT20	1.5	4.0		
D	TS-t	MT20	5.0	8.0		
E	TMVW-t	MT20	3.5	6.0	1.50	3.00
F	TMVW-t	MT20	4.0	9.0	1.50	2.75
G	BMV1+p	MT20	2.0	6.0		
H	BMVW-t	MT20	5.0	8.0	2.50	2.00
I	BMVWVW-t	MT20	3.0	12.0		
J	BS-t	MT18HS	3.25	12.0		
K	BMVW-t	MT20	5.0	8.0	2.50	2.00
L	BMV1+p	MT20	2.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2353	2353	0	1-8
JT HORZ	0	0	0	5-8
L	2353	2353	0	0
G	2353	2353	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
L	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
G	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED (LC)	MEMB.
FR-TO		FROM TO	LENGTH	FR-TO		LENGTH	
L-A	-2297 / 0	0.0	0.0 0.27 (1)	6.35	A-K	0 / 4949	0.58 (1)
A-B	-4583 / 0	-139.4	-139.4 0.44 (1)	4.22	K-B	-1720 / 0	0.33 (1)
B-C	-5888 / 0	-139.4	-139.4 0.49 (1)	3.77	B-I	0 / 1412	0.32 (1)
C-D	-5888 / 0	-139.4	-139.4 0.49 (1)	3.77	I-C	-958 / 0	0.18 (1)
D-E	-5888 / 0	-139.4	-139.4 0.49 (1)	3.77	E-I	0 / 1412	0.32 (1)
E-F	-4583 / 0	-139.4	-139.4 0.44 (1)	4.22	H-E	-1720 / 0	0.33 (1)
G-F	-2297 / 0	0.0	0.0 0.27 (1)	6.35	H-F	0 / 4949	0.58 (1)
L-K	0 / 0	-17.5	-17.5 0.20 (4)	10.00			
K-J	0 / 4583	-17.5	-17.5 0.62 (1)	10.00			
J-I	0 / 4583	-17.5	-17.5 0.62 (1)	10.00			
I-H	0 / 4583	-17.5	-17.5 0.62 (1)	10.00			
H-G	0 / 0	-17.5	-17.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 987 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 610 (0.59")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.62/1.00 (H-I:1) , WB=0.58/1.00 (A-K:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873
MT18HS	586	403	2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

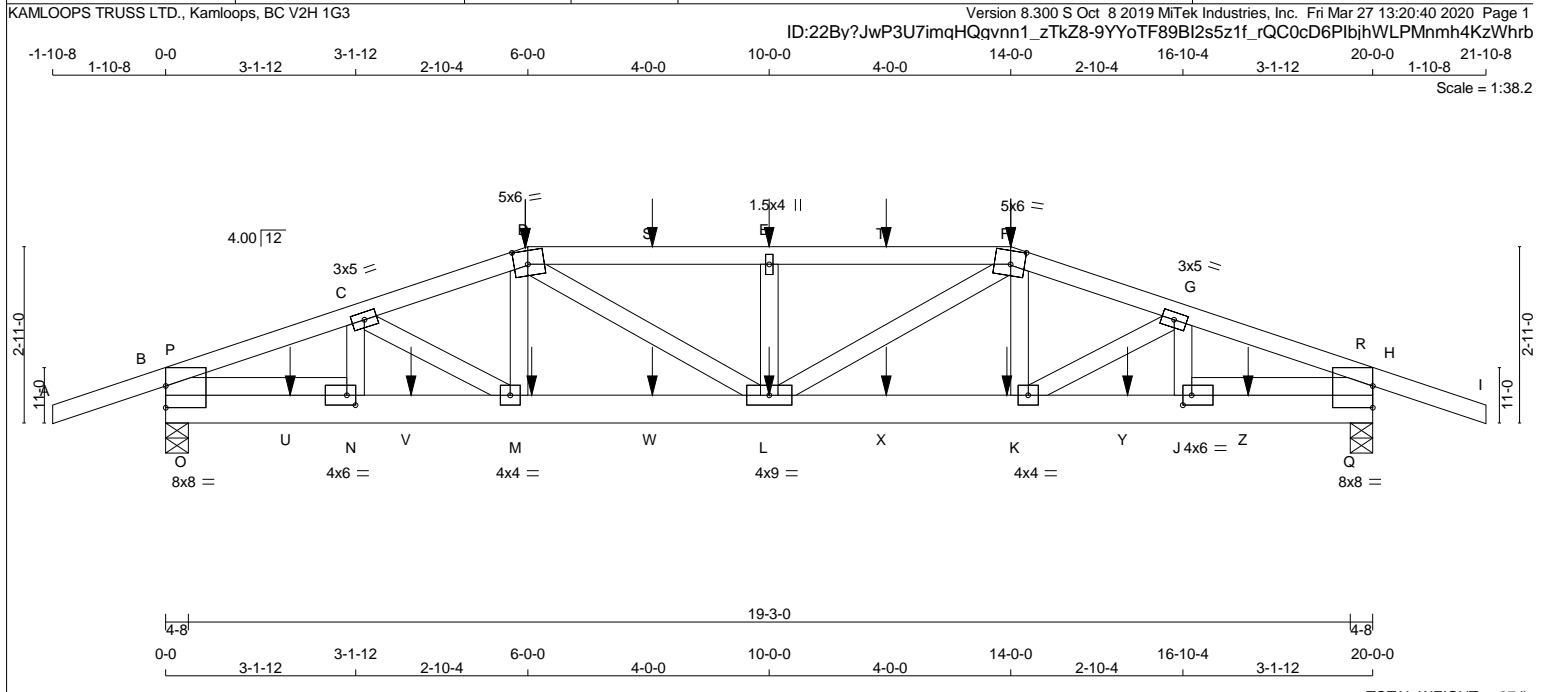
JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.88 (J) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 97 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - F	2x4	DRY 1650F 1.5E	SPF
F - I	2x4	DRY 1650F 1.5E	SPF
B - H	2x6	DRY 1650F 1.5E	SPF

REINFORCING MEMBERS

HW1	2x4	DRY 1650F 1.5E	SPF
HW2	2x4	DRY 1650F 1.5E	SPF

ALL WEBS 2x4 DRY 1650F 1.5E SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	8.0	8.0	4.25	
C	TMWW-t	MT20	3.0	5.0		
D	TTWW-m	MT20	5.0	6.0	2.75	2.75
E	TMWW+w	MT20	1.5	4.0		
F	TTWW-m	MT20	5.0	6.0	2.75	2.75
G	TMWW-t	MT20	3.0	5.0		
H	TMBMW1-I	MT20	8.0	8.0	4.25	
J	BMWW-t	MT20	4.0	6.0	2.00	1.75
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	2689	0	2689	0	4-8	2-15
H	2689	0	2689	0	4-8	2-15

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
B	1861	1447 / 0	0 / 0	0 / 0	0 / 0	414 / 0	0 / 0
H	1861	1447 / 0	0 / 0	0 / 0	0 / 0	414 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIVE (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 7	-139.4 -139.4	0.32 (1)	10.00	N-C	-748 / 0	0.05 (1)
B-P	-3124 / 0	-139.4 -139.4	0.20 (1)	4.27	C-M	0 / 451	0.06 (1)
P-C	-4412 / 0	-139.4 -139.4	0.27 (1)	3.63	M-D	0 / 110	0.02 (4)
C-D	-4821 / 0	-139.4 -139.4	0.28 (1)	3.48	D-L	0 / 1027	0.13 (1)
D-S	-5432 / 0	-139.4 -139.4	0.68 (1)	2.81	L-E	-1076 / 0	0.09 (1)
S-E	-5432 / 0	-139.4 -139.4	0.68 (1)	2.81	L-F	0 / 1027	0.13 (1)
E-T	-5432 / 0	-139.4 -139.4	0.68 (1)	2.81	K-F	0 / 110	0.02 (4)
T-F	-5432 / 0	-139.4 -139.4	0.68 (1)	2.81	K-G	0 / 451	0.06 (1)
F-G	-4821 / 0	-139.4 -139.4	0.28 (1)	3.48	J-G	-748 / 0	0.05 (1)
G-R	-4412 / 0	-139.4 -139.4	0.27 (1)	3.63	O-P	0 / 487	0.00 (1)
R-H	-3124 / 0	-139.4 -139.4	0.20 (1)	4.27	P-N	0 / 2702	0.35 (1)
H-I	0 / 7	-139.4 -139.4	0.32 (1)	10.00	Q-R	0 / 487	0.00 (1)
					R-J	0 / 2702	0.35 (1)
B-O	0 / 1543	-17.5 -17.5	0.18 (1)	10.00			
O-U	0 / 1543	-17.5 -17.5	0.25 (1)	10.00			
U-N	0 / 1543	-17.5 -17.5	0.25 (1)	10.00			
N-V	0 / 4178	-17.5 -17.5	0.47 (1)	10.00			
V-M	0 / 4178	-17.5 -17.5	0.47 (1)	10.00			
M-W	0 / 4565	-17.5 -17.5	0.46 (1)	10.00			
W-L	0 / 4565	-17.5 -17.5	0.46 (1)	10.00			
L-X	0 / 4565	-17.5 -17.5	0.46 (1)	10.00			
X-K	0 / 4565	-17.5 -17.5	0.46 (1)	10.00			
K-Y	0 / 4178	-17.5 -17.5	0.47 (1)	10.00			
Y-J	0 / 4178	-17.5 -17.5	0.47 (1)	10.00			
J-Z	0 / 1542	-17.5 -17.5	0.25 (1)	10.00			
Z-Q	0 / 1542	-17.5 -17.5	0.25 (1)	10.00			
Q-H	0 / 1542	-17.5 -17.5	0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
 ALLOWABLE DEFL.(TL)= L/360 (0.67")
 CALCULATED VERT. DEFL.(TL) = L/826 (0.29")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.47/1.00 (M-N:1)
 , WB=0.35/1.00 (J-R:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
 JSI METAL= 0.68 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME G1	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 6 of 75
--------------------	------------------	---------------	----------	--------------------------------	---------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:20:40 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-9YYoTF89BI2s5z1f_rQC0cD6PIbjhWLPmnh4KzWhrb

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-0-0	-483	-483	---	FRONT	VERT	TOTAL	---	C1
E	10-0-0	-174	-174	---	FRONT	VERT	TOTAL	---	C1
F	14-0-0	-483	-483	---	FRONT	VERT	TOTAL	---	C1
K	13-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
L	10-0-0	-25	-25	---	FRONT	VERT	TOTAL	---	C1
M	6-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
S	8-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1
T	11-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
U	2-0-12	-20	-22	---	FRONT	VERT	TOTAL	---	C1
V	4-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
W	8-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
Y	15-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
Z	17-11-4	-20	-22	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

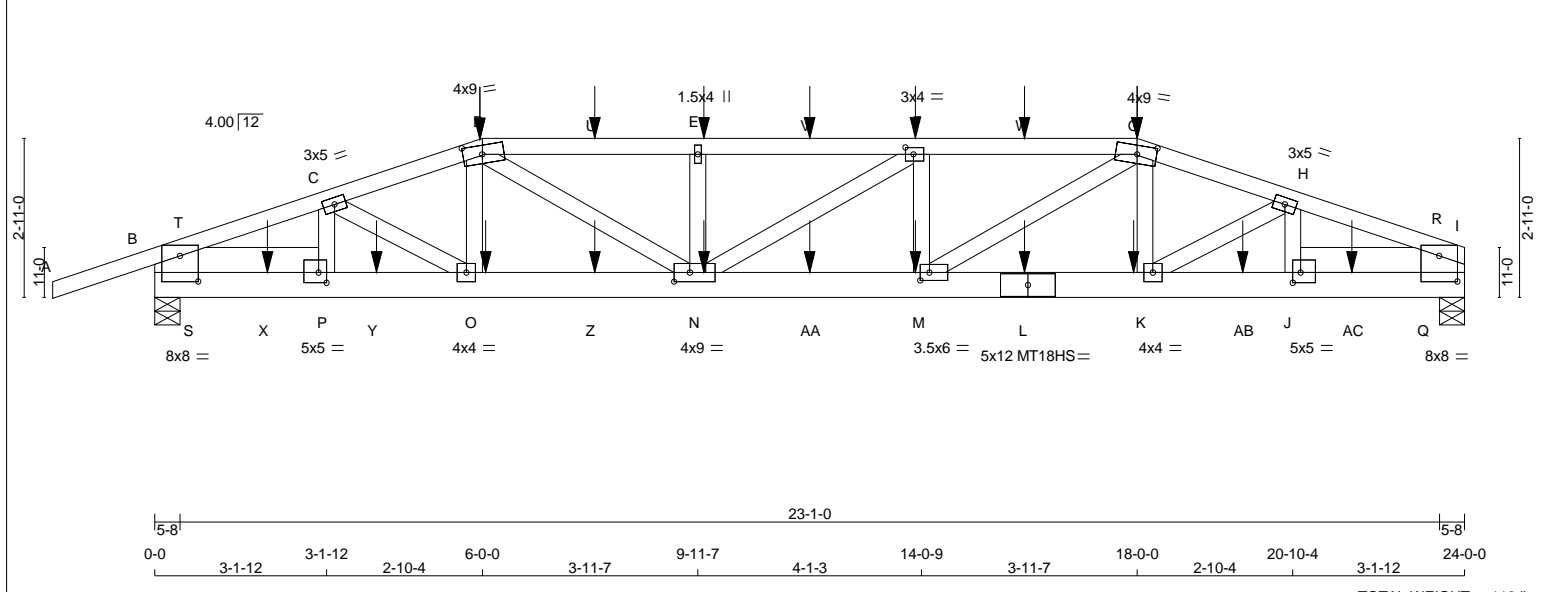
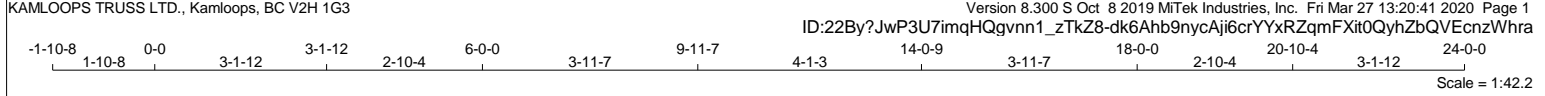
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 118 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - G	2x4	DRY 1650F 1.5E	SPF
G - I	2x4	DRY 1650F 1.5E	SPF
B - L	2x6	DRY 1650F 1.5E	SPF
L - I	2x6	DRY 1650F 1.5E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	LUMBER	DESCR.
HW1	2x6	DRY	1650F 1.5E	SPF
HW2	2x6	DRY	1650F 1.5E	SPF

ALL WEBS 2x4 DRY SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	2934	0	2934	0	5-8	3-3
B	3201	0	3201	0	5-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
I	2035	1560 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0
B	2217	1722 / 0	0 / 0	0 / 0	0 / 0	494 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, B

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MEMB.
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 7	-139.4	-139.4 0.32 (1)	10.00	P-C	-900 / 0	0.07 (1)
B-T	-3716 / 0	-139.4	-139.4 0.11 (1)	4.08	C-O	0 / 626	0.08 (1)
T-C	-5507 / 0	-139.4	-139.4 0.24 (1)	3.31	O-D	-75 / 87	0.02 (4)
C-D	-6088 / 0	-139.4	-139.4 0.36 (1)	3.05	D-N	0 / 1930	0.25 (1)
D-U	-7396 / 0	-139.4	-139.4 0.78 (1)	2.28	N-E	-969 / 0	0.08 (1)
U-E	-7396 / 0	-139.4	-139.4 0.78 (1)	2.28	N-F	-43 / 0	0.01 (1)
E-V	-7396 / 0	-139.4	-139.4 0.76 (1)	2.34	M-F	-959 / 0	0.08 (1)
V-F	-7396 / 0	-139.4	-139.4 0.76 (1)	2.34	M-G	0 / 1982	0.25 (1)
F-W	-7432 / 0	-139.4	-139.4 0.77 (1)	2.30	K-G	-102 / 83	0.02 (4)
W-G	-7432 / 0	-139.4	-139.4 0.77 (1)	2.30	K-H	0 / 613	0.08 (1)
G-H	-6079 / 0	-139.4	-139.4 0.36 (1)	3.05	J-H	-889 / 0	0.06 (1)
H-R	-5510 / 0	-139.4	-139.4 0.24 (1)	3.31	Q-R	-67 / 30	0.00 (1)
R-I	-3714 / 0	-139.4	-139.4 0.11 (1)	4.09	R-J	0 / 2844	0.23 (1)
					S-T	-63 / 30	0.00 (1)
B-S	0 / 2516	-17.5	-17.5 0.26 (1)	10.00	T-P	0 / 2839	0.23 (1)
S-X	0 / 2516	-17.5	-17.5 0.31 (1)	10.00			
X-P	0 / 2516	-17.5	-17.5 0.31 (1)	10.00			
P-Y	0 / 5227	-17.5	-17.5 0.53 (1)	10.00			
Y-O	0 / 5227	-17.5	-17.5 0.53 (1)	10.00			
O-Z	0 / 5772	-17.5	-17.5 0.57 (1)	10.00			
Z-N	0 / 5772	-17.5	-17.5 0.57 (1)	10.00			
N-AA	0 / 7433	-17.5	-17.5 0.72 (1)	10.00			
AA-M	0 / 7433	-17.5	-17.5 0.72 (1)	10.00			
M-L	0 / 5765	-17.5	-17.5 0.54 (1)	10.00			
L-K	0 / 5765	-17.5	-17.5 0.54 (1)	10.00			
K-AB	0 / 5230	-17.5	-17.5 0.53 (1)	10.00			
AB-J	0 / 5230	-17.5	-17.5 0.53 (1)	10.00			
J-AC	0 / 2514	-17.5	-17.5 0.31 (1)	10.00			
AC-Q	0 / 2514	-17.5	-17.5 0.31 (1)	10.00			
Q-I	0 / 2514	-17.5	-17.5 0.26 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(LL) = L/ 901 (0.32")
 ALLOWABLE DEFL.(TL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(TL) = L/ 553 (0.52")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.72/1.00 (M-N:1)
 , WB=0.25/1.00 (G-M:1) , SSI=0.38/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	788
MT18HS	586	403	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
 JSI METAL= 0.85 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	8.0	8.0	5.75	4.00
C	TMWW-t	MT20	3.0	5.0		
D	TTWW-m	MT20	4.0	9.0	2.00	4.25
E	TMW+w	MT20	1.5	4.0		
F	TMWW-t	MT20	3.0	4.0	1.50	1.75
G	TTWW-m	MT20	4.0	9.0	2.00	4.25
H	TMWW-t	MT20	3.0	5.0		
I	TMBMW1-I	MT20	8.0	8.0	5.75	4.00
J	BMWW-t	MT20	5.0	5.0	2.25	1.75
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT18HS	5.0	12.0		
M	BMWW-t	MT20	3.5	6.0	1.75	2.00
N	BMWW-t	MT20	4.0	9.0	2.00	3.50
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	5.0	2.25	1.75



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-0-0	-483	-483	---	FRONT	VERT	TOTAL	---	C1
E	10-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1
F	13-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
G	18-0-0	-483	-483	---	FRONT	VERT	TOTAL	---	C1
K	17-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
L	15-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
M	13-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
N	10-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
O	6-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
U	8-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1
V	12-0-0	-174	-174	---	FRONT	VERT	TOTAL	---	C1
W	15-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
X	2-0-12	-20	-22	---	FRONT	VERT	TOTAL	---	C1
Y	4-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
Z	8-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AA	12-0-0	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AC	21-11-4	-20	-22	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

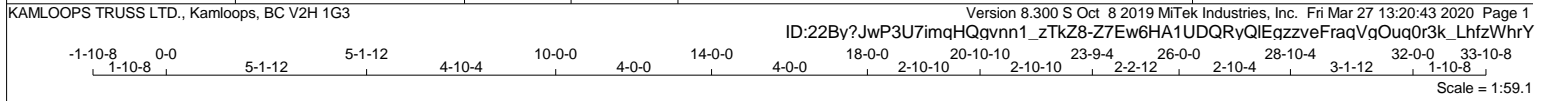
- 1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 162 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - E	2x4	DRY 1650F 1.5E	SPF
E - F	2x4	DRY 1650F 1.5E	SPF
F - H	2x4	DRY 1650F 1.5E	SPF
H - K	2x4	DRY 1650F 1.5E	SPF
B - Q	2x6	DRY 1650F 1.5E	SPF
Q - J	2x6	DRY 1650F 1.5E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	LUMBER	DESCR.
2x6	2x4	DRY 1650F 1.5E	SPF	
2x4	2x4	DRY 1650F 1.5E	SPF	

ALL WEBS 2x4 DRY 1650F 1.5E SPF
 DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
B	1875	0	1875	0	5-8	2-1		
N	4294	0	4294	0	5-8	5-8		
J	368	0	368	0	4-8	1-8		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERM.LIVE		WIND DEAD		SOIL	
	SNOW									
B	1294	1030 / 0	0 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0	0 / 0	0 / 0
N	2973	2311 / 0	0 / 0	0 / 0	0 / 0	0 / 0	662 / 0	0 / 0	0 / 0	0 / 0
J	254	204 / 0	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0	0 / 0	0 / 0

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, J
 BEARING SIZE FACTOR = 1.15 AT JNT(S) N (BASED ON SUPPORT DEPTH = 1-8)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m MT20	6.0	8.0	2.25	1.00
C	TMWW-t MT20	3.0	4.0	1.50	1.50
D	TTWW+m MT20	5.0	5.0	1.75	2.25
E	TTW-m MT20	4.0	4.0	1.75	1.75
F	TTWWW-m MT20	4.0	9.0	2.25	4.00
G	TMWW-t MT20	4.0	6.0	1.50	2.75
H	TTWW-m MT20	4.0	6.0	1.75	2.25
I	TMWW-t MT20	3.0	5.0		
J	TMBMW1-l MT20	6.0	6.0	3.75	
L	BMWW-t MT20	4.0	4.0		
M	BMWW-t MT20	3.0	5.0		
N	BMWW1-t MT20	8.0	8.0	Edge	3.50
O	BMWW-t MT20	5.0	6.0	2.25	3.00
P	BMW+w MT20	1.5	4.0		

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.02 FT OR RIGID CEILING DIRECTLY APPLIED.

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

LOADING
 TOTAL LOAD CASES: (4)

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

MEMB.	CHORDS		FACTORED				WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. CSI (LC)	
FR-TO			FROM	TO	LENGTH	FR-TO		
A-B	0 / 7	-139.4	-139.4	0.32 (1)	10.00	T-C	-176 / 40 0.01 (1)	
B-V	-2169 / 0	-139.4	-139.4	0.20 (1)	4.95	C-S	-750 / 0 0.17 (1)	
V-C	-3001 / 0	-139.4	-139.4	0.50 (1)	3.99	S-D	0 / 429 0.05 (1)	
C-D	-2311 / 0	-139.4	-139.4	0.47 (1)	4.48	D-R	-415 / 0 0.12 (1)	
D-E	-1860 / 0	-139.4	-139.4	0.39 (1)	4.99	R-E	0 / 71 0.01 (4)	
E-F	-1953 / 0	-139.4	-139.4	0.39 (1)	4.89	R-F	0 / 582 0.07 (1)	
F-G	0 / 414	-139.4	-139.4	0.64 (1)	10.00	P-F	0 / 66 0.01 (4)	
G-H	0 / 2118	-139.4	-139.4	0.86 (1)	10.00	F-O	-2378 / 0 0.33 (1)	
H-I	0 / 965	-139.4	-139.4	0.28 (1)	10.00	O-G	0 / 2271 0.29 (1)	
I-X	0 / 221	-139.4	-139.4	0.19 (1)	10.00	N-G	-2591 / 0 0.22 (1)	
X-J	-56 / 0	-139.4	-139.4	0.07 (1)	6.25	N-H	-1770 / 0 0.19 (1)	
J-K	0 / 7	-139.4	-139.4	0.32 (1)	10.00	M-H	0 / 376 0.05 (1)	
						M-I	-841 / 0 0.08 (1)	
B-U	0 / 1555	-17.5	-17.5	0.14 (1)	10.00	L-I	0 / 202 0.03 (1)	
U-T	0 / 1555	-17.5	-17.5	0.18 (1)	10.00	U-V	0 / 69 0.00 (1)	
T-S	0 / 2859	-17.5	-17.5	0.28 (1)	10.00	V-T	0 / 1323 0.11 (1)	
S-R	0 / 2166	-17.5	-17.5	0.19 (1)	10.00	L-X	-284 / 0 0.03 (1)	
R-Q	0 / 1376	-17.5	-17.5	0.14 (1)	10.00	W-X	-82 / 43 0.00 (1)	
Q-P	0 / 1376	-17.5	-17.5	0.14 (1)	10.00			
P-O	0 / 1372	-17.5	-17.5	0.15 (1)	10.00			
O-N	-2118 / 0	-17.5	-17.5	0.15 (1)	6.02			
N-M	-950 / 0	-17.5	-17.5	0.14 (1)	6.25			
M-Y	-202 / 0	-17.5	-17.5	0.03 (1)	6.25			
Y-L	-202 / 0	-17.5	-17.5	0.03 (1)	6.25			
L-Z	0 / 74	-17.5	-17.5	0.03 (4)	10.00			
Z-W	0 / 74	-17.5	-17.5	0.03 (4)	10.00			
W-J	0 / 74	-17.5	-17.5	0.02 (1)	10.00			

ALLOWABLE DEFL.(LL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
 ALLOWABLE DEFL.(TL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.86/1.00 (G-H:1) , BC=0.28/1.00 (S-T:1)
 , WB=0.33/1.00 (F-O:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
 JSI METAL= 0.79 (O) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

<p>PLATES (table is in inches)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> </thead> <tbody> <tr> <td>Q</td> <td>BS-t</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>R</td> <td>BMWW-t</td> <td>MT20</td> <td>4.0</td> <td>9.0</td> <td></td> <td></td> </tr> <tr> <td>S</td> <td>BMWW-t</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>T</td> <td>BMWW-t</td> <td>MT20</td> <td>3.5</td> <td>6.0</td> <td>2.25</td> <td>1.75</td> </tr> </tbody> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</p>	JT	TYPE	PLATES	W	LEN	Y	X	Q	BS-t	MT20	5.0	8.0			R	BMWW-t	MT20	4.0	9.0			S	BMWW-t	MT20	4.0	4.0			T	BMWW-t	MT20	3.5	6.0	2.25	1.75	<p>FACTORED CONCENTRATED LOADS (LBS)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>24-0-12</td> <td>-238</td> <td>-238</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> <tr> <td>H</td> <td>26-0-0</td> <td>-546</td> <td>-546</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> <tr> <td>M</td> <td>25-11-4</td> <td>-76</td> <td>-76</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> <tr> <td>N</td> <td>24-0-12</td> <td>-76</td> <td>-76</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> <tr> <td>Y</td> <td>27-11-4</td> <td>-25</td> <td>-25</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> <tr> <td>Z</td> <td>29-11-4</td> <td>-20</td> <td>-22</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> </tbody> </table> <p>CONNECTION REQUIREMENTS</p> <p>1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.</p>	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	G	24-0-12	-238	-238	---	FRONT	VERT	TOTAL	---	C1	H	26-0-0	-546	-546	---	FRONT	VERT	TOTAL	---	C1	M	25-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1	N	24-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1	Y	27-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1	Z	29-11-4	-20	-22	---	FRONT	VERT	TOTAL	---	C1
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																				
Q	BS-t	MT20	5.0	8.0																																																																																																						
R	BMWW-t	MT20	4.0	9.0																																																																																																						
S	BMWW-t	MT20	4.0	4.0																																																																																																						
T	BMWW-t	MT20	3.5	6.0	2.25	1.75																																																																																																				
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																	
G	24-0-12	-238	-238	---	FRONT	VERT	TOTAL	---	C1																																																																																																	
H	26-0-0	-546	-546	---	FRONT	VERT	TOTAL	---	C1																																																																																																	
M	25-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1																																																																																																	
N	24-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1																																																																																																	
Y	27-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																	
Z	29-11-4	-20	-22	---	FRONT	VERT	TOTAL	---	C1																																																																																																	

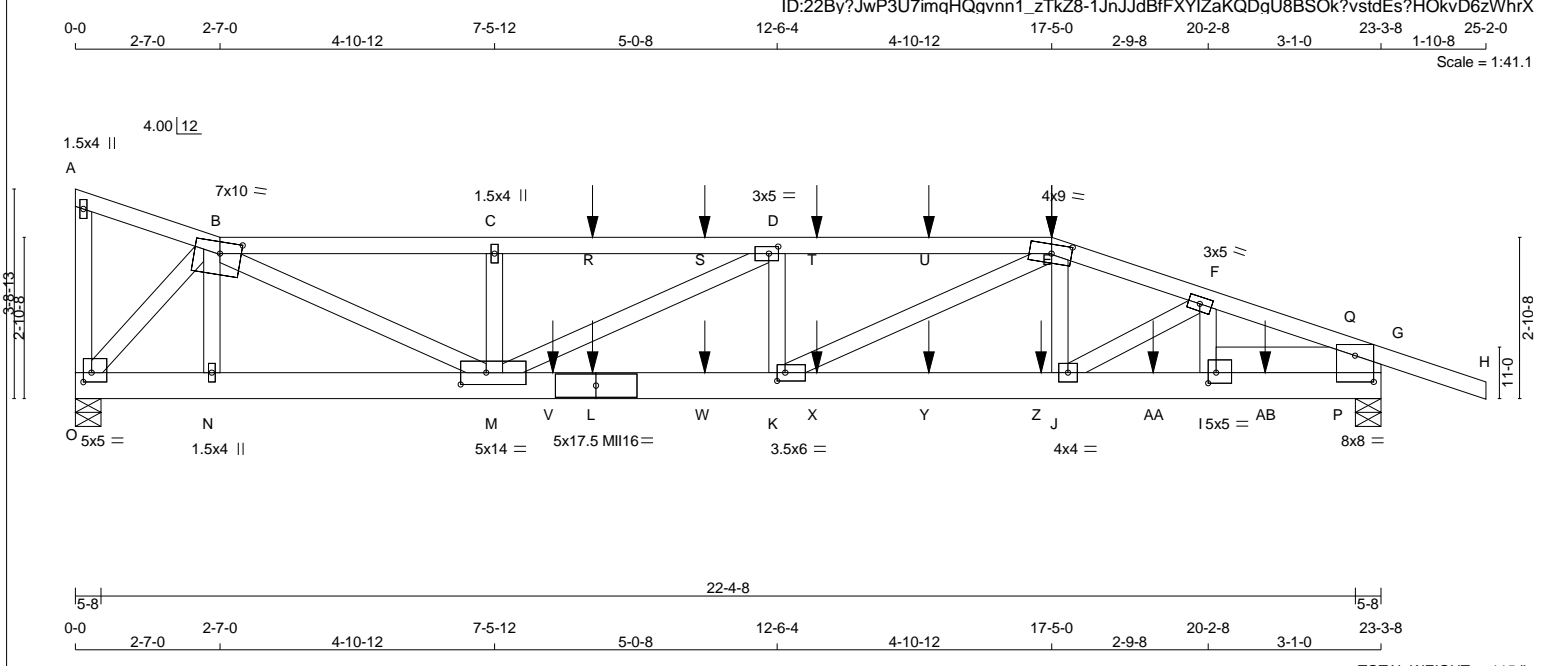


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:20:44 2020 Page 1 ID:22By?JwP3U7imqHQvnn1_zTkZ8-1JnJdBFXYIZaKQdQJ8BSOk?vstdEs?HOkvD6zWhrX



TOTAL WEIGHT = 115 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4 DRY	1650F 1.5E	SPF
A - B	2x4 DRY	1650F 1.5E	SPF
B - E	2x4 DRY	2100F 1.8E	SPF
E - H	2x4 DRY	1650F 1.5E	SPF
O - L	2x6 DRY	1650F 1.5E	SPF
L - G	2x6 DRY	1650F 1.5E	SPF

REINFORCING MEMBERS

HW2	2x6 DRY	1650F 1.5E	SPF
-----	---------	------------	-----

ALL WEBS 2x4 DRY SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	1.5	4.0		
B	TTWWW-m	MT20	7.0	10.0	2.50	4.50
C	TMW+w	MT20	1.5	4.0		
D	TMWW-t	MT20	3.0	5.0	1.50	2.00
E	TTWW-m	MT20	4.0	9.0	2.00	4.25
F	TMWW-t	MT20	3.0	5.0		
G	TMBMW-1	MT20	8.0	8.0	5.75	4.00
I	BMWW-t	MT20	5.0	5.0	2.25	1.75
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	3.5	6.0	1.75	1.75
L	BS-t	MI16	5.0	17.5		
M	BMWWW-t	MT20	5.0	14.0	2.50	5.50
N	BMW+w	MT20	1.5	4.0		
O	BMVW-t	MT20	5.0	5.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	2655	0	2655	0	5-8	2-14
G	3102	0	3102	0	5-8	3-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERMLIVE	WIND	DEAD	SOIL
	SNOW	SNOW	LIVE	PERMLIVE				
O	1836	1437 / 0	0 / 0	0 / 0	0 / 0	399 / 0	0 / 0	0 / 0
G	2146	1677 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC (LC)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC (LC)
FR-TO		FROM TO		LENGTH FR-TO				
O-A	-180 / 0	0.0	0.0	0.03 (1)	7.81	O-B	-3534 / 0	0.38 (1)
A-B	0 / 0	-139.4	-139.4	0.15 (1)	10.00	N-B	0 / 78	0.02 (4)
B-C	-6344 / 0	-139.4	-139.4	0.65 (1)	2.95	B-M	0 / 4272	0.55 (1)
C-R	-6344 / 0	-139.4	-139.4	0.87 (1)	2.61	M-C	-879 / 0	0.07 (1)
R-S	-6344 / 0	-139.4	-139.4	0.87 (1)	2.61	M-D	-1169 / 0	0.32 (1)
S-D	-6344 / 0	-139.4	-139.4	0.87 (1)	2.61	K-D	-708 / 0	0.06 (1)
D-T	-7391 / 0	-139.4	-139.4	0.96 (1)	2.27	K-E	0 / 2084	0.27 (1)
T-U	-7391 / 0	-139.4	-139.4	0.96 (1)	2.27	J-E	-111 / 93	0.02 (4)
U-E	-7391 / 0	-139.4	-139.4	0.96 (1)	2.27	J-F	0 / 602	0.08 (1)
E-F	-5834 / 0	-139.4	-139.4	0.33 (1)	3.14	I-F	-873 / 0	0.06 (1)
F-Q	-5278 / 0	-139.4	-139.4	0.22 (1)	3.40	I-Q	0 / 2719	0.22 (1)
Q-G	-3575 / 0	-139.4	-139.4	0.10 (1)	4.16	P-Q	-72 / 30	0.00 (1)
G-H	0 / 7	-139.4	-139.4	0.32 (1)	10.00			
O-N	0 / 2541	-17.5	-17.5	0.21 (1)	10.00			
N-M	0 / 2538	-17.5	-17.5	0.23 (1)	10.00			
M-V	0 / 7391	-17.5	-17.5	0.84 (1)	10.00			
V-L	0 / 7391	-17.5	-17.5	0.84 (1)	10.00			
L-W	0 / 7391	-17.5	-17.5	0.84 (1)	10.00			
W-K	0 / 7391	-17.5	-17.5	0.84 (1)	10.00			
K-X	0 / 5534	-17.5	-17.5	0.51 (1)	10.00			
X-Y	0 / 5534	-17.5	-17.5	0.51 (1)	10.00			
Y-Z	0 / 5534	-17.5	-17.5	0.51 (1)	10.00			
Z-J	0 / 5534	-17.5	-17.5	0.51 (1)	10.00			
J-AA	0 / 5009	-17.5	-17.5	0.51 (1)	10.00			
AA-I	0 / 5009	-17.5	-17.5	0.51 (1)	10.00			
I-AB	0 / 2419	-17.5	-17.5	0.30 (1)	10.00			
AB-P	0 / 2419	-17.5	-17.5	0.30 (1)	10.00			
P-G	0 / 2419	-17.5	-17.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/972 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/601 (0.47")

CSI: TC=0.96/1.00 (D-E:1), BC=0.84/1.00 (K-M:1), WB=0.55/1.00 (B-M:1), SSI=0.57/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873
MI16	438	302	2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.88 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-5-0	-479	-479	---	FRONT	VERT	TOTAL	---	C1
L	9-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
R	9-2-12	-168	-168	---	FRONT	VERT	TOTAL	---	C1
S	11-2-12	-168	-168	---	FRONT	VERT	TOTAL	---	C1
T	13-2-12	-168	-168	---	FRONT	VERT	TOTAL	---	C1
U	15-2-12	-168	-168	---	FRONT	VERT	TOTAL	---	C1
V	8-6-4	-521	-521	---	FRONT	VERT	TOTAL	---	C1
W	11-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
X	13-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
Y	15-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
Z	17-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
AA	19-2-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
AB	21-2-12	-19	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

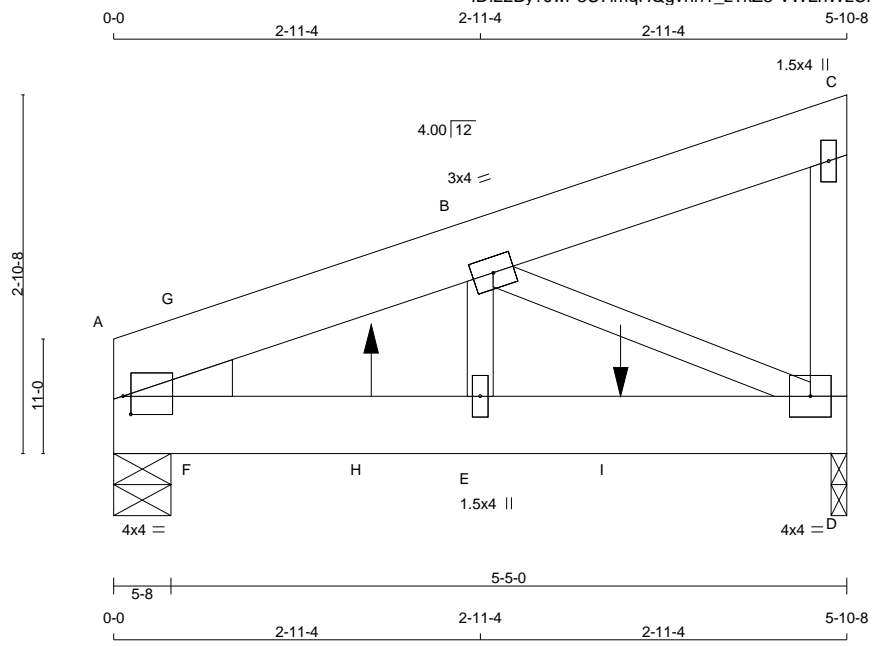
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, alexandria, VA 22314.



Scale = 1:18.5

TOTAL WEIGHT = 28 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	1650F 1.5E	SPF
D - C	2x4 DRY	1650F 1.5E	SPF
A - D	2x6 DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	4.0	1.75	0.75
B	TMWW-t	MT20	3.0	4.0		
C	TMV+p	MT20	1.5	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMW+w	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	HORZ		DOWN	HORZ	UPLIFT			
D	526	0		526	0	0	1-8	1-8	
A	479	0		479	0	0	5-8	1-8	2x4 L

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
D	362	293 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
A	330	268 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

MEMB.	C H O R D S			W E B S			
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-G	-690 / 0	-139.4	-139.4 0.03 (1)	6.25	E-B	-8 / 65	0.02 (1)
G-B	-564 / 0	-139.4	-139.4 0.03 (5)	6.25	B-D	-593 / 0	0.12 (1)
B-C	-6 / 0	-139.4	-139.4 0.07 (1)	10.00	F-G	0 / 54	0.00 (1)
D-C	-189 / 0	0.0	0.0 0.02 (1)	7.81			

A-F	0 / 534	-17.5	-17.5 0.05 (1)	10.00
F-H	0 / 534	-17.5	-17.5 0.06 (5)	10.00
H-E	0 / 534	-17.5	-17.5 0.06 (5)	10.00
E-I	0 / 534	-17.5	-17.5 0.10 (1)	10.00
I-D	0 / 534	-17.5	-17.5 0.10 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	2-0-12	21	1	28	BACK	VERT	TOTAL	---	C1
I	4-0-12	-105	-105	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.07/1.00 (B-C:1), BC=0.10/1.00 (D-E:1), WB=0.12/1.00 (B-D:1), SSI=0.14/1.00 (B-C:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

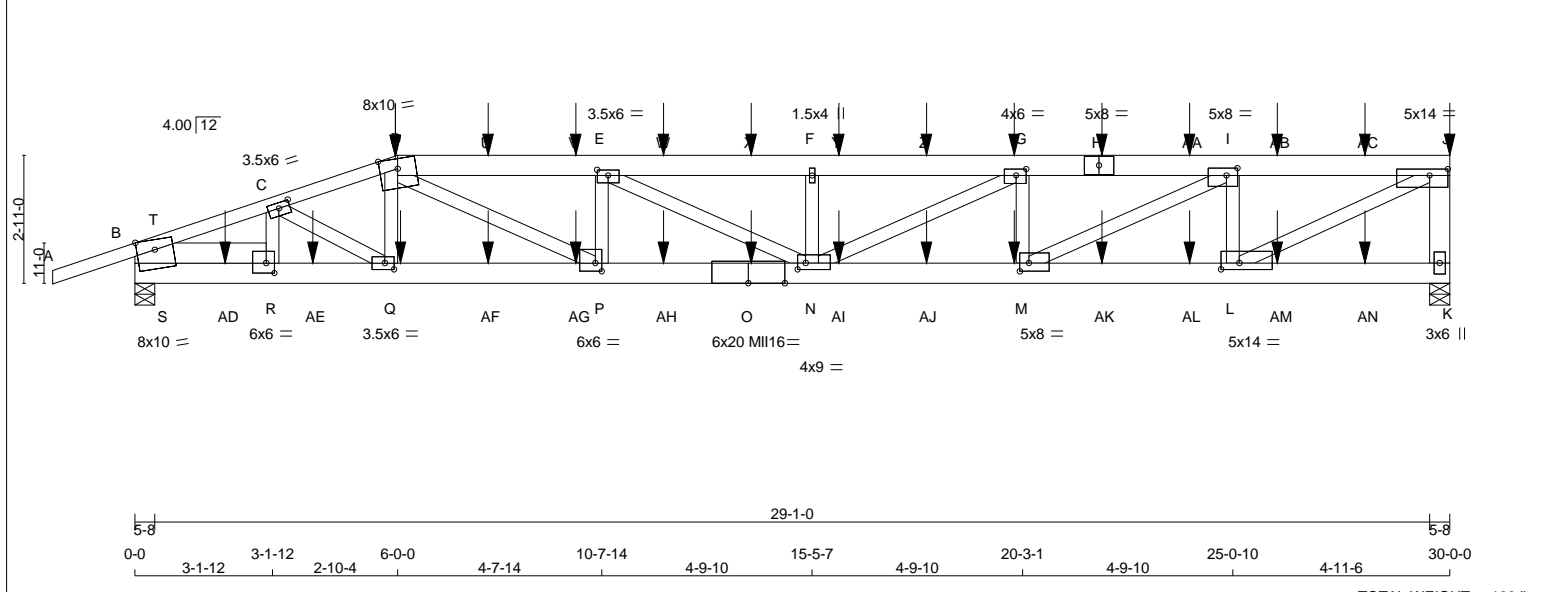
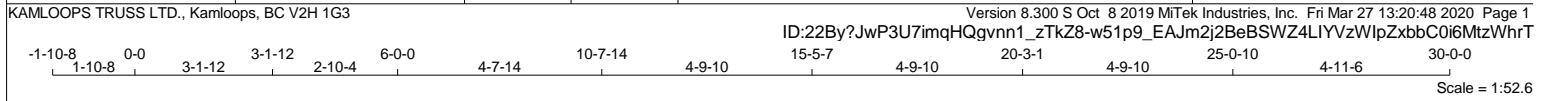
JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.19 (A) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - H	2x6	DRY 1650F 1.5E	SPF
H - J	2x6	DRY 1650F 1.5E	SPF
K - J	2x6	DRY 1650F 1.5E	SPF
B - O	2x6	DRY 2400F 2.0E	DF
O - K	2x6	DRY 2400F 2.0E	DF

REINFORCING MEMBERS

HW1	2x6	DRY 1650F 1.5E	SPF
-----	-----	----------------	-----

ALL WEBS 2x4 DRY SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m	MT20	8.0	10.0	Edge	
C	TMWW-t	MT20	3.5	6.0	1.50	3.00
D	TTWW-m	MT20	8.0	10.0	Edge	
E	TMWW-t	MT20	3.5	6.0	1.50	3.00
F	TMW+w	MT20	1.5	4.0		
G	TMWW-t	MT20	4.0	6.0	1.75	2.75
H	TS-t	MT20	5.0	8.0		
I	TMWW-t	MT20	5.0	8.0	2.00	3.00
J	TMV-t	MT20	5.0	14.0	1.75	5.00
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	14.0	1.75	5.00
M	BMWW-t	MT20	5.0	8.0	2.25	2.50
N	BMWWW-t	MT20	4.0	9.0	1.75	2.25
O	BS-t	MI16	6.0	20.0		
P	BMWW-t	MT20	6.0	6.0	2.25	1.75
Q	BMWW-t	MT20	3.5	6.0	1.75	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
K	4020	0	0	5-8
B	3941	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
K	2782	2172 / 0	0 / 0	0 / 0	0 / 0	610 / 0	0 / 0
B	2729	2119 / 0	0 / 0	0 / 0	0 / 0	610 / 0	0 / 0

BEARING MATERIAL TO BE DF NO.2 OR BETTER AT JOINT(S) K, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S		W E B S	
MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED FR-TO LENGTH (FT)
A-B	0 / 7	-139.4 -139.4	0.32 (1) 10.00
B-T	-4687 / 0	-139.4 -139.4	0.16 (1) 3.65
T-C	-6974 / 0	-139.4 -139.4	0.34 (1) 2.87
C-D	-8019 / 0	-139.4 -139.4	0.51 (1) 2.50
D-U	-10983 / 0	-139.4 -139.4	0.60 (1) 2.67
U-V	-10983 / 0	-139.4 -139.4	0.60 (1) 2.67
V-E	-10983 / 0	-139.4 -139.4	0.60 (1) 2.67
E-W	-11835 / 0	-139.4 -139.4	0.61 (1) 2.58
W-X	-11835 / 0	-139.4 -139.4	0.61 (1) 2.58
X-F	-11835 / 0	-139.4 -139.4	0.61 (1) 2.58
F-Y	-11835 / 0	-139.4 -139.4	0.64 (1) 2.55
Y-Z	-11835 / 0	-139.4 -139.4	0.64 (1) 2.55
Z-G	-11835 / 0	-139.4 -139.4	0.64 (1) 2.55
G-H	-10422 / 0	-139.4 -139.4	0.54 (1) 2.81
H-AA	-10422 / 0	-139.4 -139.4	0.54 (1) 2.81
AA-I	-10422 / 0	-139.4 -139.4	0.54 (1) 2.81
I-AB	-6623 / 0	-139.4 -139.4	0.40 (1) 3.61
AB-AC	-6623 / 0	-139.4 -139.4	0.40 (1) 3.61
AC-J	-6623 / 0	-139.4 -139.4	0.40 (1) 3.61
K-J	-3932 / 0	0.0	0.0 0.23 (1) 6.10

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
DL = 5.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 627 (0.57")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 386 (0.93")

CSI: TC=0.64/1.00 (F-G:1) , BC=0.58/1.00 (N-P:1)
.WB=0.95/1.00 (J-L:1) , SS=0.42/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873
MI16	438	302	2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.500 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.93 (B) (INPUT = 1.00)

This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

CONTINUED ON PAGE 2

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 Mitek Industries, Inc. Fri Mar 27 13:20:48 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-w51p9_EAJm2j2BeBSWZ4LIYVzWlpZxbbC0i6MtZWhrT

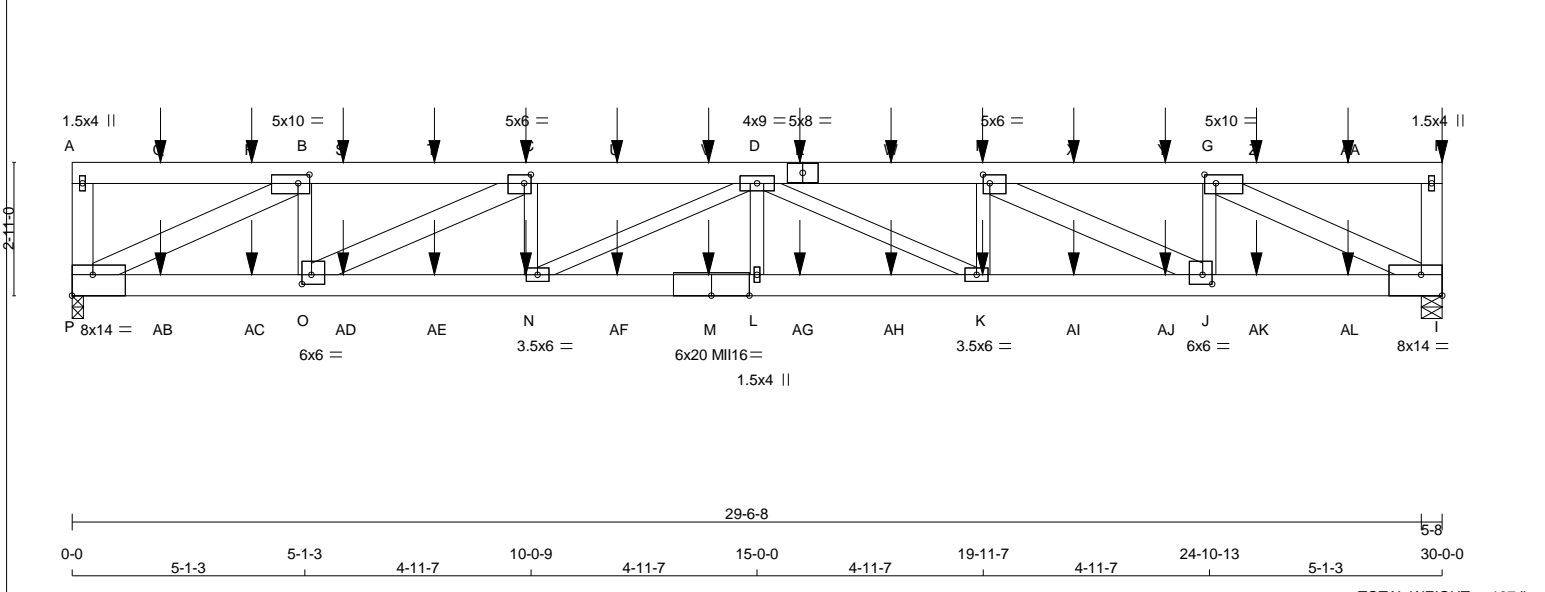
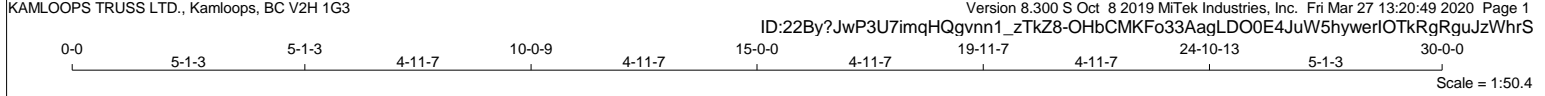
<p>PLATES (table is in inches)</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td>JT TYPE</td> <td>PLATES</td> <td>W</td> <td>LEN</td> <td>Y</td> <td>X</td> </tr> <tr> <td>R</td> <td>BMW-t</td> <td>MT20</td> <td>6.0</td> <td>6.0</td> <td>2.75 2.25</td> </tr> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</p>	JT TYPE	PLATES	W	LEN	Y	X	R	BMW-t	MT20	6.0	6.0	2.75 2.25	<p>LOADING</p> <p>TOTAL LOAD CASES: (4)</p> <table style="width:100%; border-collapse: collapse;"> <tr> <th colspan="4">C H O R D S</th> <th colspan="4">W E B S</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1 MAX CSI (LC)</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1 MAX CSI (LC)</th> </tr> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td>LENGTH</td> <td>FR-TO</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AH-O</td> <td>0 / 10982</td> <td>-17.5</td> <td>-17.5 0.58 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>O-N</td> <td>0 / 10982</td> <td>-17.5</td> <td>-17.5 0.58 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>N-AI</td> <td>0 / 10422</td> <td>-17.5</td> <td>-17.5 0.55 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AI-AJ</td> <td>0 / 10422</td> <td>-17.5</td> <td>-17.5 0.55 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AJ-M</td> <td>0 / 10422</td> <td>-17.5</td> <td>-17.5 0.55 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>M-AK</td> <td>0 / 6623</td> <td>-17.5</td> <td>-17.5 0.37 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AK-AL</td> <td>0 / 6623</td> <td>-17.5</td> <td>-17.5 0.37 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AL-L</td> <td>0 / 6623</td> <td>-17.5</td> <td>-17.5 0.37 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>L-AM</td> <td>0 / 0</td> <td>-17.5</td> <td>-17.5 0.06 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AM-AN</td> <td>0 / 0</td> <td>-17.5</td> <td>-17.5 0.06 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AN-K</td> <td>0 / 0</td> <td>-17.5</td> <td>-17.5 0.06 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> </table> <p>FACTORED CONCENTRATED LOADS (LBS)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr><td>D</td><td>6-0-0</td><td>-485</td><td>-485</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>G</td><td>20-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>H</td><td>22-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>J</td><td>30-0-0</td><td>-240</td><td>-240</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>M</td><td>20-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>O</td><td>14-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>Q</td><td>6-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>U</td><td>8-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>V</td><td>10-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>W</td><td>12-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>X</td><td>14-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>Y</td><td>16-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>Z</td><td>18-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AA</td><td>24-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AB</td><td>26-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AC</td><td>28-0-12</td><td>-174</td><td>-174</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AD</td><td>2-0-12</td><td>-20</td><td>-22</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AE</td><td>4-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AF</td><td>8-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AG</td><td>10-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AH</td><td>12-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AI</td><td>16-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AJ</td><td>18-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AK</td><td>22-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AL</td><td>24-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AM</td><td>26-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>AN</td><td>28-0-12</td><td>-25</td><td>-25</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> </tbody> </table> <p>CONNECTION REQUIREMENTS</p> <p>1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.</p>	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	FR-TO		FROM TO	LENGTH	FR-TO				AH-O	0 / 10982	-17.5	-17.5 0.58 (1)	10.00				O-N	0 / 10982	-17.5	-17.5 0.58 (1)	10.00				N-AI	0 / 10422	-17.5	-17.5 0.55 (1)	10.00				AI-AJ	0 / 10422	-17.5	-17.5 0.55 (1)	10.00				AJ-M	0 / 10422	-17.5	-17.5 0.55 (1)	10.00				M-AK	0 / 6623	-17.5	-17.5 0.37 (1)	10.00				AK-AL	0 / 6623	-17.5	-17.5 0.37 (1)	10.00				AL-L	0 / 6623	-17.5	-17.5 0.37 (1)	10.00				L-AM	0 / 0	-17.5	-17.5 0.06 (1)	10.00				AM-AN	0 / 0	-17.5	-17.5 0.06 (1)	10.00				AN-K	0 / 0	-17.5	-17.5 0.06 (1)	10.00				JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	D	6-0-0	-485	-485	---	FRONT	VERT	TOTAL	---	C1	G	20-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	H	22-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	J	30-0-0	-240	-240	---	FRONT	VERT	TOTAL	---	C1	M	20-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	O	14-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	Q	6-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	U	8-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	V	10-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	W	12-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	X	14-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	Y	16-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	Z	18-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	AA	24-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	AB	26-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	AC	28-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1	AD	2-0-12	-20	-22	---	FRONT	VERT	TOTAL	---	C1	AE	4-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AF	8-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AG	10-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AH	12-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AI	16-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AJ	18-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AK	22-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AL	24-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AM	26-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1	AN	28-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
JT TYPE	PLATES	W	LEN	Y	X																																																																																																																																																																																																																																																																																																																																																																																																																
R	BMW-t	MT20	6.0	6.0	2.75 2.25																																																																																																																																																																																																																																																																																																																																																																																																																
C H O R D S				W E B S																																																																																																																																																																																																																																																																																																																																																																																																																	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)																																																																																																																																																																																																																																																																																																																																																																																																														
FR-TO		FROM TO	LENGTH	FR-TO																																																																																																																																																																																																																																																																																																																																																																																																																	
AH-O	0 / 10982	-17.5	-17.5 0.58 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
O-N	0 / 10982	-17.5	-17.5 0.58 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
N-AI	0 / 10422	-17.5	-17.5 0.55 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AI-AJ	0 / 10422	-17.5	-17.5 0.55 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AJ-M	0 / 10422	-17.5	-17.5 0.55 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
M-AK	0 / 6623	-17.5	-17.5 0.37 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AK-AL	0 / 6623	-17.5	-17.5 0.37 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AL-L	0 / 6623	-17.5	-17.5 0.37 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
L-AM	0 / 0	-17.5	-17.5 0.06 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AM-AN	0 / 0	-17.5	-17.5 0.06 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
AN-K	0 / 0	-17.5	-17.5 0.06 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																	
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																																																																																																																																																																																																																																																																																																																												
D	6-0-0	-485	-485	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
G	20-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
H	22-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
J	30-0-0	-240	-240	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
M	20-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
O	14-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
Q	6-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
U	8-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
V	10-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
W	12-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
X	14-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
Y	16-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
Z	18-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AA	24-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AB	26-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AC	28-0-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AD	2-0-12	-20	-22	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AE	4-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AF	8-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AG	10-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AH	12-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AI	16-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AJ	18-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AK	22-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AL	24-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AM	26-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												
AN	28-0-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																												



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.com and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 187 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x6	DRY 1650F 1.5E	SPF
A - E	2x6	DRY 1650F 1.5E	SPF
E - H	2x6	DRY 1650F 1.5E	SPF
I - H	2x6	DRY 1650F 1.5E	SPF
P - M	2x6	DRY 2400F 2.0E	DF
M - I	2x6	DRY 2400F 2.0E	DF
ALL WEBS 2x4 DRY 1650F 1.5E SPF			
EXCEPT			
P - B	2x6	DRY 2400F 2.0E	DF
O - C	2x6	DRY 1650F 1.5E	SPF
F - J	2x6	DRY 1650F 1.5E	SPF
G - I	2x6	DRY 2400F 2.0E	DF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	1.5	4.0		
B	TMWW-t	MT20	5.0	10.0	2.25	3.00
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TMWWW-t	MT20	4.0	9.0		
E	TS-t	MT20	5.0	8.0		
F	TMWW-t	MT20	5.0	6.0	2.25	1.75
G	TMWW-t	MT20	5.0	10.0	2.25	3.00
H	TMV+p	MT20	1.5	4.0		
I	BMVV-t	MT20	8.0	14.0	5.50	Edge
J	BMWW-t	MT20	6.0	6.0	2.50	2.50
K	BMWW-t	MT20	3.5	6.0		
L	BMWW+w	MT20	1.5	4.0		
M	BS-t	MI16	6.0	20.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	3753	3753	0	3-0
JT HORZ	0	0	0	3-0
I VERT	3983	3983	0	5-8
I HORZ	0	0	0	3-4

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT COMBINED	2600	2012 / 0	0 / 0
P	2756	2152 / 0	0 / 0

BEARING MATERIAL TO BE DF NO.2 OR BETTER AT JOINT(S) I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	C H O R D S		W E B S	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO				
P-A	-402 / 0	0.0	0.0	7.81
A-Q	0 / 0	-139.4	-139.4	0.26 (1)
Q-R	0 / 0	-139.4	-139.4	0.26 (1)
R-B	0 / 0	-139.4	-139.4	0.26 (1)
B-S	-6782 / 0	-139.4	-139.4	0.42 (1)
S-T	-6782 / 0	-139.4	-139.4	0.42 (1)
T-C	-6782 / 0	-139.4	-139.4	0.42 (1)
C-U	-10494 / 0	-139.4	-139.4	0.56 (1)
U-V	-10494 / 0	-139.4	-139.4	0.56 (1)
V-D	-10494 / 0	-139.4	-139.4	0.56 (1)
D-E	-10498 / 0	-139.4	-139.4	0.56 (1)
E-W	-10498 / 0	-139.4	-139.4	0.56 (1)
W-F	-10498 / 0	-139.4	-139.4	0.56 (1)
F-X	-6784 / 0	-139.4	-139.4	0.42 (1)
X-Y	-6784 / 0	-139.4	-139.4	0.42 (1)
Y-G	-6784 / 0	-139.4	-139.4	0.42 (1)
G-Z	0 / 0	-139.4	-139.4	0.26 (1)
Z-AA	0 / 0	-139.4	-139.4	0.26 (1)
AA-H	0 / 0	-139.4	-139.4	0.26 (1)
I-H	-632 / 0	0.0	0.0	0.04 (1)
P-AB	0 / 6782	-17.5	-17.5	0.34 (1)
AB-AC	0 / 6782	-17.5	-17.5	0.34 (1)
AC-O	0 / 6782	-17.5	-17.5	0.34 (1)
O-AD	0 / 10494	-17.5	-17.5	0.52 (1)
AD-AE	0 / 10494	-17.5	-17.5	0.52 (1)
AE-N	0 / 10494	-17.5	-17.5	0.52 (1)
N-AF	0 / 11785	-17.5	-17.5	0.59 (1)
AF-M	0 / 11785	-17.5	-17.5	0.59 (1)
M-L	0 / 11785	-17.5	-17.5	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/748 (0.48")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/459 (0.78")

CSI: TC=0.56/1.00 (D-F:1), BC=0.59/1.00 (K-L:1), WB=0.91/1.00 (G-I:1), SSI=0.44/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
MI16	438	302	2547

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
 JSI METAL= 0.97 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

ID:22By?JwP3U7ImqHQgvnn1_zTkZ8-OHbCMKFo33AagLDO0E4JuW5hywerlOTkRgRguJzWhrS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWW-t	MT20	3.5	6.0		
O	BMWW-t	MT20	6.0	6.0	2.50	2.50
P	BMWW1-t	MT20	8.0	14.0	5.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
L-AG	0 / 11785	-17.5	-17.5	0.59 (1)	10.00		
AG-AH	0 / 11785	-17.5	-17.5	0.59 (1)	10.00		
AH-K	0 / 11785	-17.5	-17.5	0.59 (1)	10.00		
K-AI	0 / 10498	-17.5	-17.5	0.52 (1)	10.00		
AI-AJ	0 / 10498	-17.5	-17.5	0.52 (1)	10.00		
AJ-J	0 / 10498	-17.5	-17.5	0.52 (1)	10.00		
J-AK	0 / 6784	-17.5	-17.5	0.34 (1)	10.00		
AK-AL	0 / 6784	-17.5	-17.5	0.34 (1)	10.00		
AL-I	0 / 6784	-17.5	-17.5	0.34 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	9-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
E	15-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
F	19-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
H	30-0-0	-240	-240	---	FRONT	VERT	TOTAL	---	C1
K	19-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
M	13-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
N	9-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
Q	1-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
S	5-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
U	11-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
W	17-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
X	21-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
Y	23-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
Z	25-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
AA	27-11-4	-174	-174	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AD	5-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AE	7-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AF	11-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AG	15-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AH	17-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AI	21-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AJ	23-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AK	25-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AL	27-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

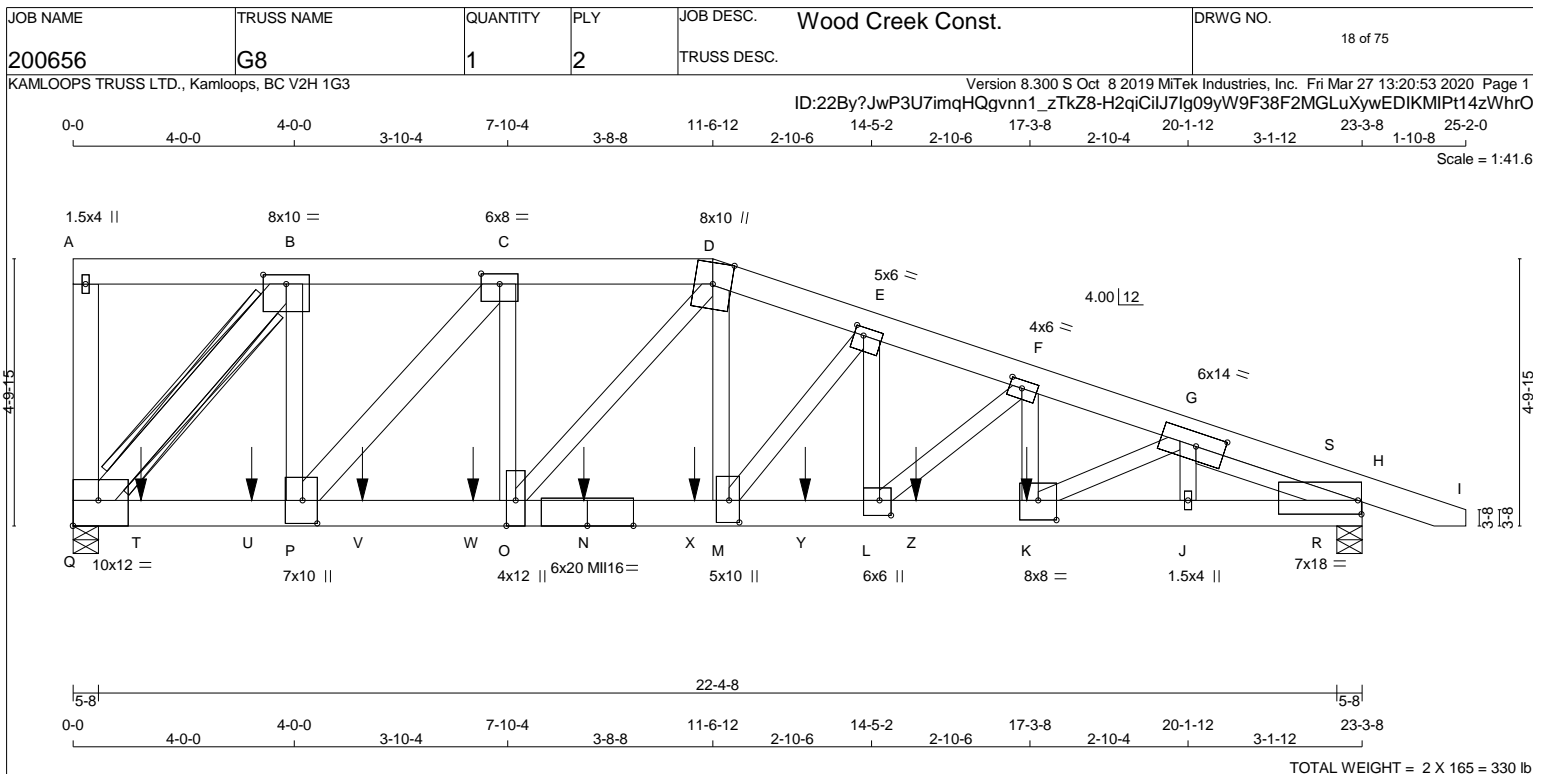
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
Q - A	2x6 DRY	1650F 1.5E	SPF
A - D	2x6 DRY	1650F 1.5E	SPF
D - I	2x6 DRY	1650F 1.5E	SPF
Q - N	2x6 DRY	2400F 2.0E	DF
N - H	2x6 DRY	2400F 2.0E	DF

REINFORCING MEMBERS

HW2	2x4	DRY	1650F 1.5E	SPF
ALL WEBS	2x4	DRY	1650F 1.5E	SPF
EXCEPT				
Q - B	2x6	DRY	2400F 2.0E	DF
P - C	2x6	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
Q - A	2 12	TOP
A - D	2 12	TOP
D - I	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - N	2 12	SIDE(0.0)
N - H	2 12	SIDE(203.5)
WEBS : (0.122"x3") SPIRAL NAILS		
F - K	2 2	SIDE(770.9)
2x4	1 6	
2x6	2 6	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	HORZ	UPLIFT
Q	14867	0	14867
H	11471	0	11471

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
Q	10281 8064 / 0 0 / 0 0 / 0 0 / 0 2217 / 0 0 / 0
H	7932 6228 / 0 0 / 0 0 / 0 0 / 0 1704 / 0 0 / 0

BEARING MATERIAL TO BE DF NO.2 OR BETTER AT JOINT(S) Q, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) Q (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 I-BRACE AT B-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
DL = 5.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/966 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/600 (0.47")

CSI: TC=0.63/1.00 (F-G:1) , BC=0.78/1.00 (K-L:1) , WB=0.85/1.00 (C-P:1) , SSI=0.81/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
MI16	438	302	2547
			1256
			4283
			1816

PLATE PLACEMENT TOL. = 0.500 inches EXCEPT:

BOTTOM EDGE OF PLATE AT JNT(S) Q TO BE PLACED WITHIN 0.25" OF SUPPORT SURFACE

PLATE ROTATION TOL. = 5.0 Deg.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH
FR-TO				
Q-A	-210 / 0	0.0	0.0	0.02 (1)
A-B	0 / 0	-139.4	-139.4	0.07 (1)
B-C	-11927 / 0	-139.4	-139.4	0.15 (1)
C-D	-18140 / 0	-139.4	-139.4	0.27 (1)
D-E	-21413 / 0	-139.4	-139.4	0.34 (1)
E-F	-24992 / 0	-139.4	-139.4	0.47 (1)
F-G	-27391 / 0	-139.4	-139.4	0.63 (1)
G-S	-13483 / 0	-139.4	-139.4	0.31 (1)
S-H	-16744 / 0	-139.4	-139.4	0.29 (1)
H-I	0 / 13	-139.4	-139.4	0.06 (1)

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO		
Q-B	-17660 / 0	7.81
P-B	0 / 12298	10.00
P-C	-9390 / 0	4.02
O-C	0 / 6692	3.23
O-D	-2951 / 0	2.92
M-D	0 / 8442	2.58
M-E	-5413 / 0	2.25
L-E	0 / 5007	3.64
L-F	-3057 / 0	3.32
K-F	0 / 2205	10.00
K-G	0 / 4446	10.00
J-G	-486 / 0	10.00
G-R	-11122 / 0	10.00
R-S	0 / 6030	10.00

CONTINUED ON PAGE 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

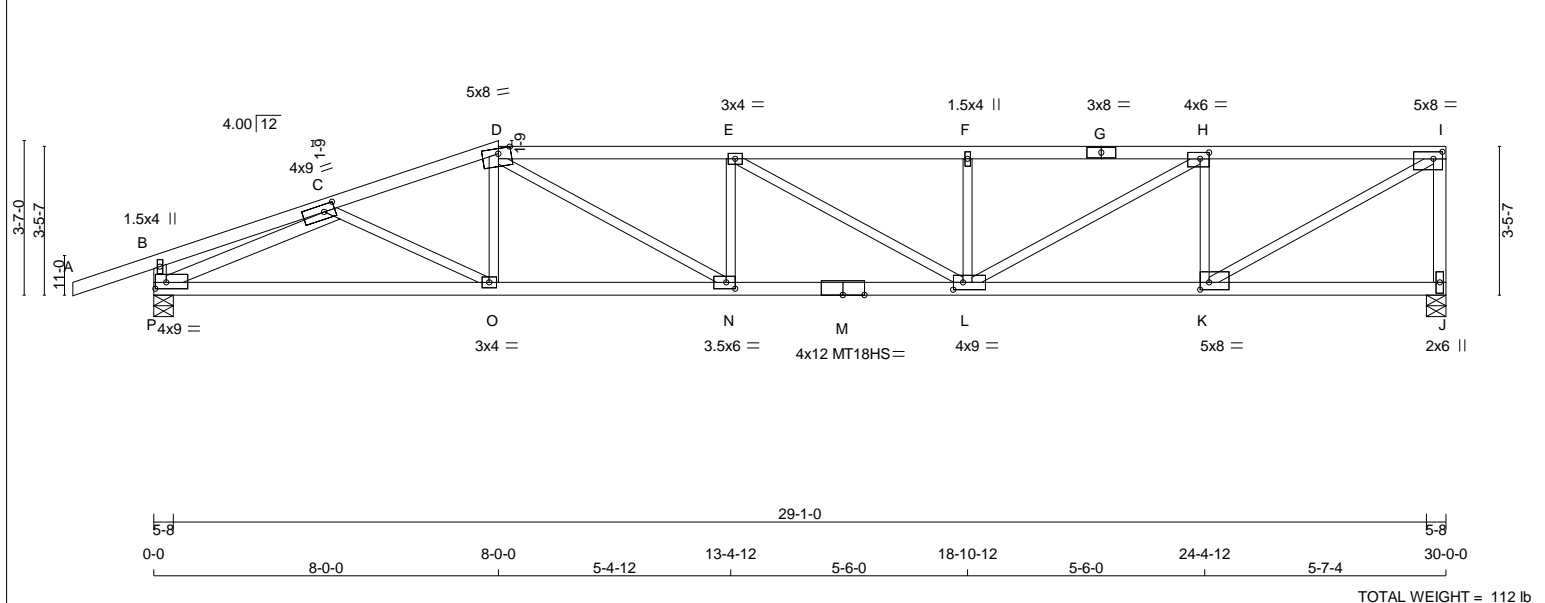
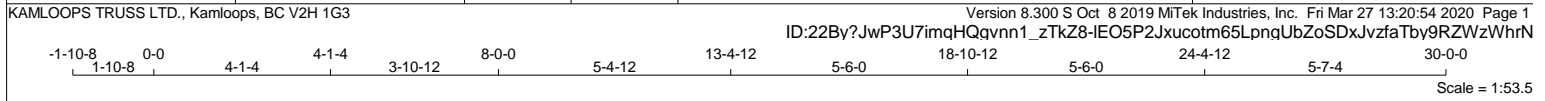
<p>NAILS TO BE DRIVEN FROM ONE SIDE ONLY.</p> <p>GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.</p> <p>PLATES (table is in inches)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> </thead> <tbody> <tr><td>A</td><td>TMV+p</td><td>MT20</td><td>1.5</td><td>4.0</td><td></td><td></td></tr> <tr><td>B</td><td>TMWW-t</td><td>MT20</td><td>8.0</td><td>10.0</td><td>2.00</td><td>5.00</td></tr> <tr><td>C</td><td>TMWW-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td>2.25</td><td>4.00</td></tr> <tr><td>D</td><td>TTWW+m</td><td>MT20</td><td>8.0</td><td>10.0</td><td>Edge</td><td></td></tr> <tr><td>E</td><td>TMWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td>1.75</td><td>2.00</td></tr> <tr><td>F</td><td>TMWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td>1.75</td><td>2.75</td></tr> <tr><td>G</td><td>TMWW-t</td><td>MT20</td><td>6.0</td><td>14.0</td><td>3.00</td><td>6.25</td></tr> <tr><td>H</td><td>TMBW1-l</td><td>MT20</td><td>7.0</td><td>18.0</td><td>3.00</td><td>0.75</td></tr> <tr><td>J</td><td>BMW+w</td><td>MT20</td><td>1.5</td><td>4.0</td><td></td><td></td></tr> <tr><td>K</td><td>BMWW-t</td><td>MT20</td><td>8.0</td><td>8.0</td><td>4.25</td><td>4.00</td></tr> <tr><td>L</td><td>BMWW+t</td><td>MT20</td><td>6.0</td><td>6.0</td><td>3.25</td><td>2.50</td></tr> <tr><td>M</td><td>BMWW+t</td><td>MT20</td><td>5.0</td><td>10.0</td><td>4.75</td><td>2.25</td></tr> <tr><td>N</td><td>BS-t</td><td>MI16</td><td>6.0</td><td>20.0</td><td></td><td></td></tr> <tr><td>O</td><td>BMWW+t</td><td>MT20</td><td>4.0</td><td>12.0</td><td>Edge</td><td></td></tr> <tr><td>P</td><td>BMWW+t</td><td>MT20</td><td>7.0</td><td>10.0</td><td>5.00</td><td>3.25</td></tr> <tr><td>Q</td><td>BMVW1-t</td><td>MT20</td><td>10.0</td><td>12.0</td><td>5.50</td><td>Edge</td></tr> </tbody> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</p>	JT	TYPE	PLATES	W	LEN	Y	X	A	TMV+p	MT20	1.5	4.0			B	TMWW-t	MT20	8.0	10.0	2.00	5.00	C	TMWW-t	MT20	6.0	8.0	2.25	4.00	D	TTWW+m	MT20	8.0	10.0	Edge		E	TMWW-t	MT20	5.0	6.0	1.75	2.00	F	TMWW-t	MT20	4.0	6.0	1.75	2.75	G	TMWW-t	MT20	6.0	14.0	3.00	6.25	H	TMBW1-l	MT20	7.0	18.0	3.00	0.75	J	BMW+w	MT20	1.5	4.0			K	BMWW-t	MT20	8.0	8.0	4.25	4.00	L	BMWW+t	MT20	6.0	6.0	3.25	2.50	M	BMWW+t	MT20	5.0	10.0	4.75	2.25	N	BS-t	MI16	6.0	20.0			O	BMWW+t	MT20	4.0	12.0	Edge		P	BMWW+t	MT20	7.0	10.0	5.00	3.25	Q	BMVW1-t	MT20	10.0	12.0	5.50	Edge	<p>LOADING</p> <p>TOTAL LOAD CASES: (4)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="4">C H O R D S</th> <th colspan="4">W E B S</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1 MAX CSI (LC)</th> <th>MAX. UNBRAC</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX CSI (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td>LENGTH</td> <td>FR-TO</td> <td></td> <td></td> </tr> <tr> <td>M-Y</td> <td>0 / 23713</td> <td>-17.5</td> <td>-17.5</td> <td>0.70 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>Y-L</td> <td>0 / 23713</td> <td>-17.5</td> <td>-17.5</td> <td>0.70 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>L-Z</td> <td>0 / 26037</td> <td>-17.5</td> <td>-17.5</td> <td>0.78 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>Z-K</td> <td>0 / 26037</td> <td>-17.5</td> <td>-17.5</td> <td>0.78 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>K-J</td> <td>0 / 22152</td> <td>-17.5</td> <td>-17.5</td> <td>0.64 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>J-R</td> <td>0 / 22119</td> <td>-17.5</td> <td>-17.5</td> <td>0.64 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>R-H</td> <td>0 / 12629</td> <td>-17.5</td> <td>-17.5</td> <td>0.29 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> </tbody> </table> <p>FACTORED CONCENTRATED LOADS (LBS)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr><td>K</td><td>17-2-12</td><td>-3735</td><td>-3735</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>N</td><td>9-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>T</td><td>1-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>U</td><td>3-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>V</td><td>5-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>W</td><td>7-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>X</td><td>11-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>Y</td><td>13-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> <tr><td>Z</td><td>15-2-12</td><td>-2336</td><td>-2336</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr> </tbody> </table> <p>CONNECTION REQUIREMENTS</p> <p>1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.</p>	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM TO		LENGTH	FR-TO			M-Y	0 / 23713	-17.5	-17.5	0.70 (1)	10.00			Y-L	0 / 23713	-17.5	-17.5	0.70 (1)	10.00			L-Z	0 / 26037	-17.5	-17.5	0.78 (1)	10.00			Z-K	0 / 26037	-17.5	-17.5	0.78 (1)	10.00			K-J	0 / 22152	-17.5	-17.5	0.64 (1)	10.00			J-R	0 / 22119	-17.5	-17.5	0.64 (1)	10.00			R-H	0 / 12629	-17.5	-17.5	0.29 (1)	10.00			JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	K	17-2-12	-3735	-3735	---	BACK	VERT	TOTAL	---	C1	N	9-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	T	1-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	U	3-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	V	5-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	W	7-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	X	11-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	Y	13-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	Z	15-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1	<p>JSI GRIP= 0.90 (G) (INPUT = 0.90) JSI METAL= 0.87 (N) (INPUT = 1.00)</p>
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																																																																																																																																																																																																							
A	TMV+p	MT20	1.5	4.0																																																																																																																																																																																																																																																																																																									
B	TMWW-t	MT20	8.0	10.0	2.00	5.00																																																																																																																																																																																																																																																																																																							
C	TMWW-t	MT20	6.0	8.0	2.25	4.00																																																																																																																																																																																																																																																																																																							
D	TTWW+m	MT20	8.0	10.0	Edge																																																																																																																																																																																																																																																																																																								
E	TMWW-t	MT20	5.0	6.0	1.75	2.00																																																																																																																																																																																																																																																																																																							
F	TMWW-t	MT20	4.0	6.0	1.75	2.75																																																																																																																																																																																																																																																																																																							
G	TMWW-t	MT20	6.0	14.0	3.00	6.25																																																																																																																																																																																																																																																																																																							
H	TMBW1-l	MT20	7.0	18.0	3.00	0.75																																																																																																																																																																																																																																																																																																							
J	BMW+w	MT20	1.5	4.0																																																																																																																																																																																																																																																																																																									
K	BMWW-t	MT20	8.0	8.0	4.25	4.00																																																																																																																																																																																																																																																																																																							
L	BMWW+t	MT20	6.0	6.0	3.25	2.50																																																																																																																																																																																																																																																																																																							
M	BMWW+t	MT20	5.0	10.0	4.75	2.25																																																																																																																																																																																																																																																																																																							
N	BS-t	MI16	6.0	20.0																																																																																																																																																																																																																																																																																																									
O	BMWW+t	MT20	4.0	12.0	Edge																																																																																																																																																																																																																																																																																																								
P	BMWW+t	MT20	7.0	10.0	5.00	3.25																																																																																																																																																																																																																																																																																																							
Q	BMVW1-t	MT20	10.0	12.0	5.50	Edge																																																																																																																																																																																																																																																																																																							
C H O R D S				W E B S																																																																																																																																																																																																																																																																																																									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)																																																																																																																																																																																																																																																																																																						
FR-TO		FROM TO		LENGTH	FR-TO																																																																																																																																																																																																																																																																																																								
M-Y	0 / 23713	-17.5	-17.5	0.70 (1)	10.00																																																																																																																																																																																																																																																																																																								
Y-L	0 / 23713	-17.5	-17.5	0.70 (1)	10.00																																																																																																																																																																																																																																																																																																								
L-Z	0 / 26037	-17.5	-17.5	0.78 (1)	10.00																																																																																																																																																																																																																																																																																																								
Z-K	0 / 26037	-17.5	-17.5	0.78 (1)	10.00																																																																																																																																																																																																																																																																																																								
K-J	0 / 22152	-17.5	-17.5	0.64 (1)	10.00																																																																																																																																																																																																																																																																																																								
J-R	0 / 22119	-17.5	-17.5	0.64 (1)	10.00																																																																																																																																																																																																																																																																																																								
R-H	0 / 12629	-17.5	-17.5	0.29 (1)	10.00																																																																																																																																																																																																																																																																																																								
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																																																																																																																																																																																																																				
K	17-2-12	-3735	-3735	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
N	9-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
T	1-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
U	3-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
V	5-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
W	7-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
X	11-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
Y	13-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				
Z	15-2-12	-2336	-2336	---	BACK	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																				



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 112 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - G	2x4	DRY 1650F 1.5E	SPF
G - I	2x4	DRY 1650F 1.5E	SPF
J - I	2x4	DRY 1650F 1.5E	SPF
P - B	2x4	DRY 1650F 1.5E	SPF
P - M	2x4	DRY 1650F 1.5E	SPF
M - J	2x4	DRY 1650F 1.5E	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
P - C	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	1.5	4.0		
C	TMWW-t	MT20	4.0	9.0	2.00	3.00
D	TTWW-m	MT20	5.0	8.0	1.50	3.50
E	TMWW-t	MT20	3.0	4.0		
F	TMW+w	MT20	1.5	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0	1.75	2.50
I	TMV-t	MT20	5.0	8.0	2.00	2.50
J	BMV1+p	MT20	2.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.00	2.50
L	BMWWW-t	MT20	4.0	9.0	2.00	2.75
M	BS-t	MT18HS	4.0	12.0		
N	BMWW-t	MT20	3.5	6.0	1.75	2.50
O	BMWW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	4.0	9.0	1.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	2353	0	2353	0	5-8	4-2
P	2621	0	2621	0	5-8	2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERMLIVE	WIND	DEAD	SOIL
	SNOW	SNOW	LIVE	PERMLIVE				
J	1629	1269 / 0	0 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
P	1811	1431 / 0	0 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-139.4	-139.4 0.30 (1)	10.00	C-O	0 / 109	0.04 (4)
B-C	0 / 16	-139.4	-139.4 0.23 (1)	10.00	O-D	0 / 110	0.04 (4)
C-D	-4536 / 0	-139.4	-139.4 0.36 (1)	3.54	D-N	0 / 1562	0.35 (1)
D-E	-5647 / 0	-139.4	-139.4 0.82 (1)	2.80	N-E	-720 / 0	0.15 (1)
E-F	-5297 / 0	-139.4	-139.4 0.80 (1)	2.92	E-L	-402 / 0	0.29 (1)
F-G	-5297 / 0	-139.4	-139.4 0.85 (1)	2.84	L-F	-715 / 0	0.14 (1)
G-H	-5297 / 0	-139.4	-139.4 0.85 (1)	2.84	L-H	0 / 2028	0.46 (1)
H-I	-3539 / 0	-139.4	-139.4 0.68 (1)	3.56	K-H	-1890 / 0	0.38 (1)
J-I	-2309 / 0	0.0	0.0 0.30 (1)	6.34	K-I	0 / 4063	0.91 (1)
P-B	-501 / 0	0.0	0.0 0.03 (1)	7.81	P-C	-4719 / 0	0.84 (1)
P-O	0 / 4246	-17.5	-17.5 0.61 (1)	10.00			
O-N	0 / 4305	-17.5	-17.5 0.58 (1)	10.00			
N-M	0 / 5646	-17.5	-17.5 0.73 (1)	10.00			
M-L	0 / 5646	-17.5	-17.5 0.73 (1)	10.00			
L-K	0 / 3539	-17.5	-17.5 0.47 (1)	10.00			
K-J	0 / 0	-17.5	-17.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/906 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/553 (0.65")

CSI: TC=0.85/1.00 (F-H:1), BC=0.73/1.00 (L-N:1), WB=0.91/1.00 (I-K:1), SSI=0.36/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873
MT18HS	586	403	2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H1	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 21 of 75	H1X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:20:54 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-IEO5P2Jxucotm65LpngUbZoSDxJvzfaTby9RZWzWhrN

JSI GRIP= 0.90 (C) (INPUT = 0.90)
 JSI METAL= 0.86 (M) (INPUT = 1.00)

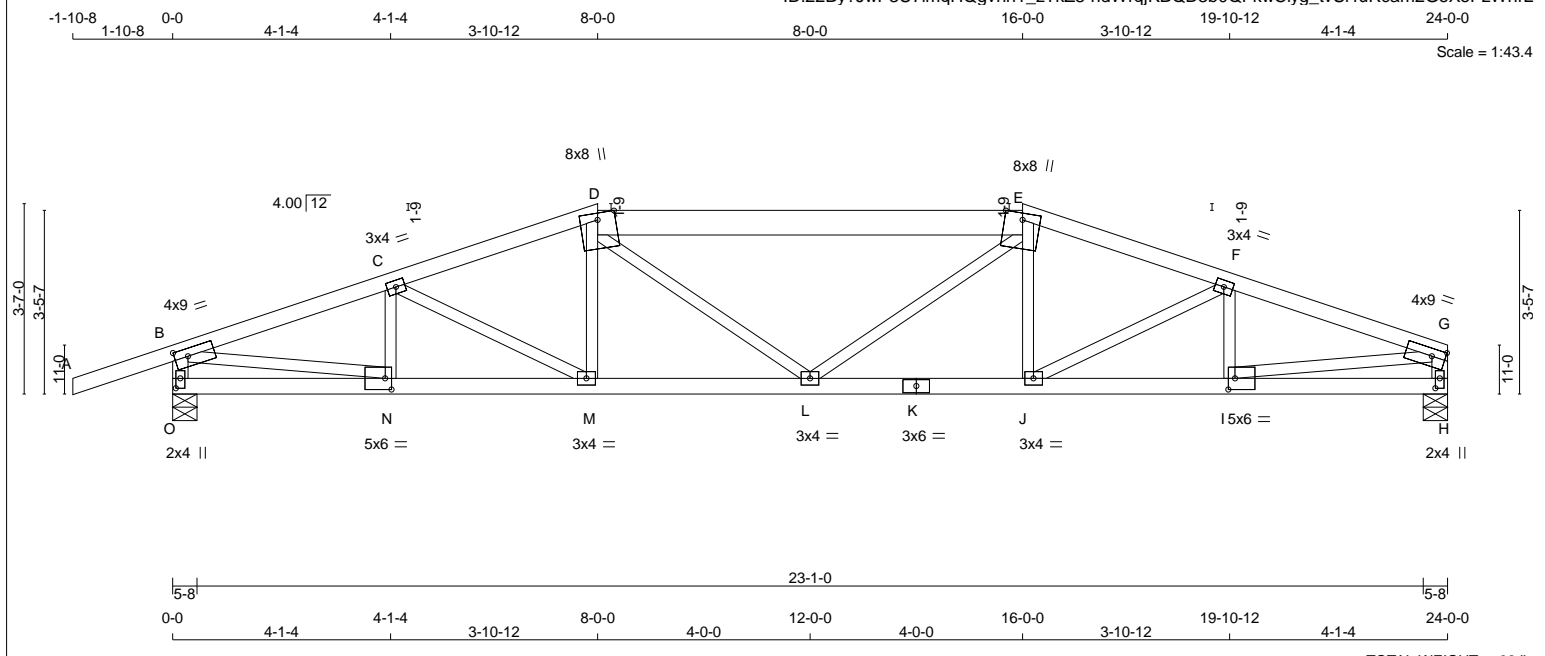


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:20:56 2020 Page 1
 ID:22By?JwP3U7imqHQgvnn1_zTkZ8-hdWrqjKBQD3b0QFkwCiyg_tvSl4uRcam2GeXePzWhrL



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x6	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
O - B	2x4	DRY	1650F 1.5E	SPF
H - G	2x4	DRY	1650F 1.5E	SPF
O - K	2x4	DRY	1650F 1.5E	SPF
K - H	2x4	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0		
D	TTWW+m	MT20	8.0	8.0	Edge	
E	TTWW+m	MT20	8.0	8.0	Edge	
F	TMVW-t	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	9.0	1.75	3.00
H	BMV1+P	MT20	2.0	4.0	2.25	1.00
I	BMVW-t	MT20	5.0	6.0	2.50	1.50
J, L, M						
J	BMVW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0	2.50	1.50
O	BMV1+P	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UP/LIFT	IN-SX
O	2150	0	0	5-8
H	1883	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
O	1485	1178 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0
H	1303	1015 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-139.4	-139.4 0.30 (1)	10.00	N-C	-453 / 0	0.07 (1)
B-C	-3478 / 0	-139.4	-139.4 0.27 (1)	4.08	C-M	-167 / 0	0.05 (1)
C-D	-3328 / 0	-139.4	-139.4 0.24 (1)	4.19	M-D	0 / 162	0.04 (1)
D-E	-3206 / 0	-139.4	-139.4 0.42 (1)	4.84	D-L	0 / 66	0.02 (4)
E-F	-3328 / 0	-139.4	-139.4 0.24 (1)	4.19	L-E	0 / 66	0.02 (4)
F-G	-3478 / 0	-139.4	-139.4 0.27 (1)	4.08	J-E	0 / 162	0.04 (1)
O-B	-2105 / 0	0.0	0.0 0.13 (1)	6.57	J-F	-168 / 0	0.05 (1)
H-G	-1837 / 0	0.0	0.0 0.12 (1)	6.92	I-F	-453 / 0	0.07 (1)
					B-N	0 / 3352	0.75 (1)
					I-G	0 / 3352	0.75 (1)
O-N	0 / 0	-17.5	-17.5 0.07 (1)	10.00			
N-M	0 / 3315	-17.5	-17.5 0.44 (1)	10.00			
M-L	0 / 3161	-17.5	-17.5 0.39 (1)	10.00			
L-K	0 / 3161	-17.5	-17.5 0.39 (1)	10.00			
K-J	0 / 3161	-17.5	-17.5 0.39 (1)	10.00			
J-I	0 / 3315	-17.5	-17.5 0.44 (1)	10.00			
I-H	0 / 0	-17.5	-17.5 0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
 ALLOWABLE DEFL.(TL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.44/1.00 (M-N:1)
 , WB=0.75/1.00 (G-I:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
 JSI METAL= 0.94 (K) (INPUT = 1.00)



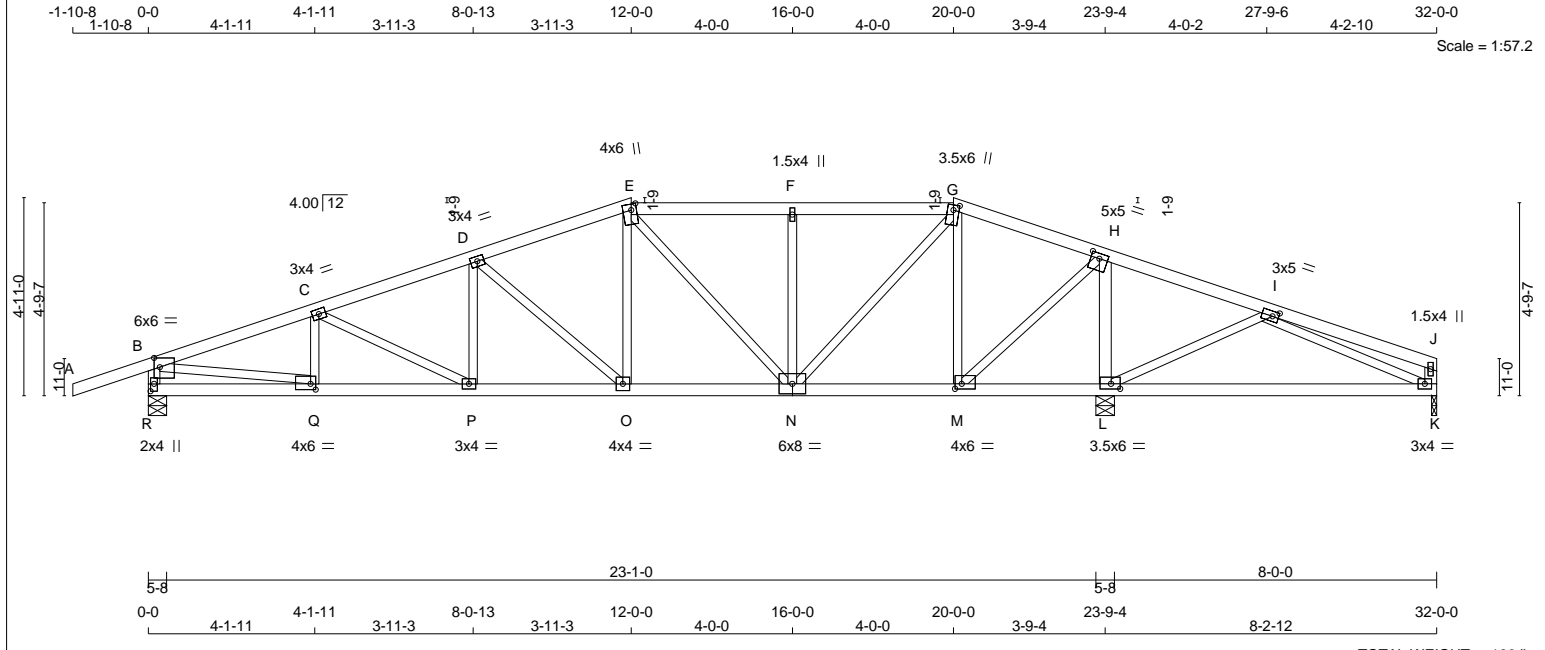
This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:20:57 2020 Page 1

ID:22By?JwP3U7imqHQgvnn1_zTKZ8-9p4D23LpBxBdaqwUvDBCCQ499PxA4LvHwN5ArzWhrk



TOTAL WEIGHT = 126 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY 1650F 1.5E	SPF
E - G	2x4	DRY 1650F 1.5E	SPF
G - J	2x4	DRY 1650F 1.5E	SPF
R - B	2x4	DRY 1650F 1.5E	SPF
K - J	2x4	DRY 1650F 1.5E	SPF
R - N	2x4	DRY 1650F 1.5E	SPF
N - K	2x4	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT L - H 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
R	1951 0	1951 0 0	5-8	2-14
L	3215 0	3215 0 0	5-8	3-10
K	122 0	122 0 0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1347	1070 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0
L	2225	1735 / 0	0 / 0	0 / 0	0 / 0	490 / 0	0 / 0
K	85	65 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX UNBRAC	MEMB. MAX FORCE (LBS) MAX CSI (LC)
FR-TO				FR-TO			
A-B	0 / 42	-139.4 -139.4	0.30 (1)	10.00	Q-C	-376 / 0	0.06 (1)
B-C	-3034 / 0	-139.4 -139.4	0.28 (1)	4.30	C-P	-297 / 0	0.10 (1)
C-D	-2767 / 0	-139.4 -139.4	0.22 (1)	4.54	P-D	0 / 209	0.05 (1)
D-E	-2086 / 0	-139.4 -139.4	0.21 (1)	5.07	D-O	-864 / 0	0.36 (1)
E-F	-1583 / 0	-139.4 -139.4	0.24 (1)	5.58	O-E	0 / 642	0.14 (1)
F-G	-1583 / 0	-139.4 -139.4	0.24 (1)	5.58	E-N	-570 / 0	0.36 (1)
G-H	-511 / 0	-139.4 -139.4	0.21 (1)	6.25	N-F	-734 / 0	0.25 (1)
H-I	0 / 1233	-139.4 -139.4	0.42 (1)	10.00	N-G	0 / 1637	0.37 (1)
I-J	0 / 18	-139.4 -139.4	0.28 (1)	10.00	M-G	-1471 / 0	0.52 (1)
R-B	-1908 / 0	0.0 0.0	0.12 (1)	6.82	M-H	0 / 2222	0.50 (1)
K-J	-232 / 0	0.0 0.0	0.01 (1)	7.81	L-H	-2649 / 0	0.28 (1)
					L-I	-922 / 0	0.33 (1)
R-Q	0 / 0	-17.5 -17.5	0.06 (1)	10.00	B-Q	0 / 2923	0.66 (1)
Q-P	0 / 2891	-17.5 -17.5	0.39 (1)	10.00	I-K	0 / 380	0.09 (1)
P-O	0 / 2627	-17.5 -17.5	0.33 (1)	10.00			
O-N	0 / 1958	-17.5 -17.5	0.26 (1)	10.00			
N-M	0 / 502	-17.5 -17.5	0.10 (1)	10.00			
M-L	-1170 / 0	-17.5 -17.5	0.18 (4)	6.25			
L-K	-342 / 0	-17.5 -17.5	0.20 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.42/1.00 (H-I:1), BC=0.39/1.00 (P-Q:1), WB=0.66/1.00 (B-Q:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.69 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H3	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 24 of 75	H3X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:20:57 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTKZ8-9p4D23LpBxBsdaqwUvDBCCQ499PxA4LvHwN5ArzWhrK

PLATES (table is in inches)

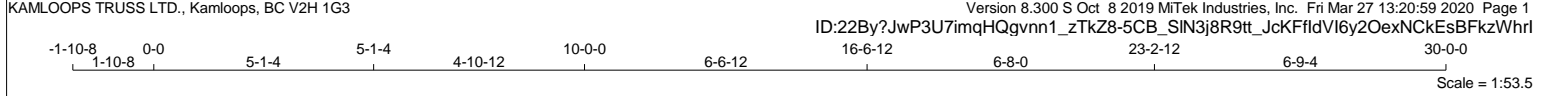
JT	TYPE	PLATES	W	LEN	Y	X
Q	BMW-t	MT20	4.0	6.0	1.75	1.50
R	BMV1+p	MT20	2.0	4.0	2.25	1.00



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - G	2x4	DRY 2100F 1.8E	SPF
H - G	2x4	DRY 1650F 1.5E	SPF
N - B	2x8	DRY 1950F 1.7E	DF
N - K	2x4	DRY 1650F 1.5E	SPF
K - H	2x4	DRY 1650F 1.5E	SPF
ALL WEBS EXCEPT B - M	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	12.0	1.75	6.00
C	TMVW-t	MT20	3.0	4.0		
D	TTWW-m	MT20	4.0	9.0	1.00	3.50
E	TMW+w	MT20	1.5	4.0		
F	TMVW-t	MT20	3.5	6.0	1.50	3.00
G	TMVW-t	MT20	5.0	8.0	2.25	2.00
H	BMV1+P	MT20	2.0	6.0		
I	BMVW-t	MT20	5.0	8.0	2.25	2.00
J	BMVWWW-t	MT20	4.0	9.0		
K	BS-t	MT18HS	3.25	12.0		
L	BMVW-t	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.25
N	BMV1+P	MT20	2.0	6.0	Edge	3.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2353	2353	0	5-8
H	2353	2353	0	4-2
N	2621	2621	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
H	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
N	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED (LC)	MEMB.
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-139.4	-139.4 0.30 (1)	10.00	M-C	-474 / 0	0.08 (1)
B-C	-4664 / 0	-139.4	-139.4 0.67 (1)	3.20	L-D	-434 / 0	0.21 (1)
C-D	-4282 / 0	-139.4	-139.4 0.62 (1)	3.35	L-D	0 / 311	0.07 (1)
D-E	-4719 / 0	-139.4	-139.4 0.84 (1)	3.29	D-J	0 / 779	0.18 (1)
E-F	-4718 / 0	-139.4	-139.4 0.90 (1)	3.20	J-E	-1046 / 0	0.27 (1)
F-G	-3399 / 0	-139.4	-139.4 0.76 (1)	3.80	J-F	0 / 1520	0.34 (1)
H-G	-2303 / 0	0.0	0.0 0.44 (1)	6.35	I-F	-1790 / 0	0.46 (1)
N-B	-2571 / 0	0.0	0.0 0.07 (1)	7.81	I-G	0 / 3904	0.88 (1)
					B-M	0 / 4479	0.52 (1)
N-M	0 / 0	-17.5	-17.5 0.09 (4)	10.00			
M-L	0 / 4447	-17.5	-17.5 0.58 (1)	10.00			
L-K	0 / 4047	-17.5	-17.5 0.53 (1)	10.00			
K-J	0 / 4047	-17.5	-17.5 0.53 (1)	10.00			
J-I	0 / 3399	-17.5	-17.5 0.46 (1)	10.00			
I-H	0 / 0	-17.5	-17.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/729 (0.49")

CSI: TC=0.90/1.00 (E-F:1) , BC=0.58/1.00 (L-M:1) , WB=0.88/1.00 (G-1:1) , SS=0.44/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873
MT18HS	586	403	2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

CONTINUED ON PAGE 2

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H4	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 26 of 75	H4X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:20:59 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-5CB_SIN3j8R9tt_JcKffldVI6y2OexNCkEsBFkzWhrl

JSI GRIP= 0.90 (D) (INPUT = 0.90)
 JSI METAL= 0.79 (M) (INPUT = 1.00)

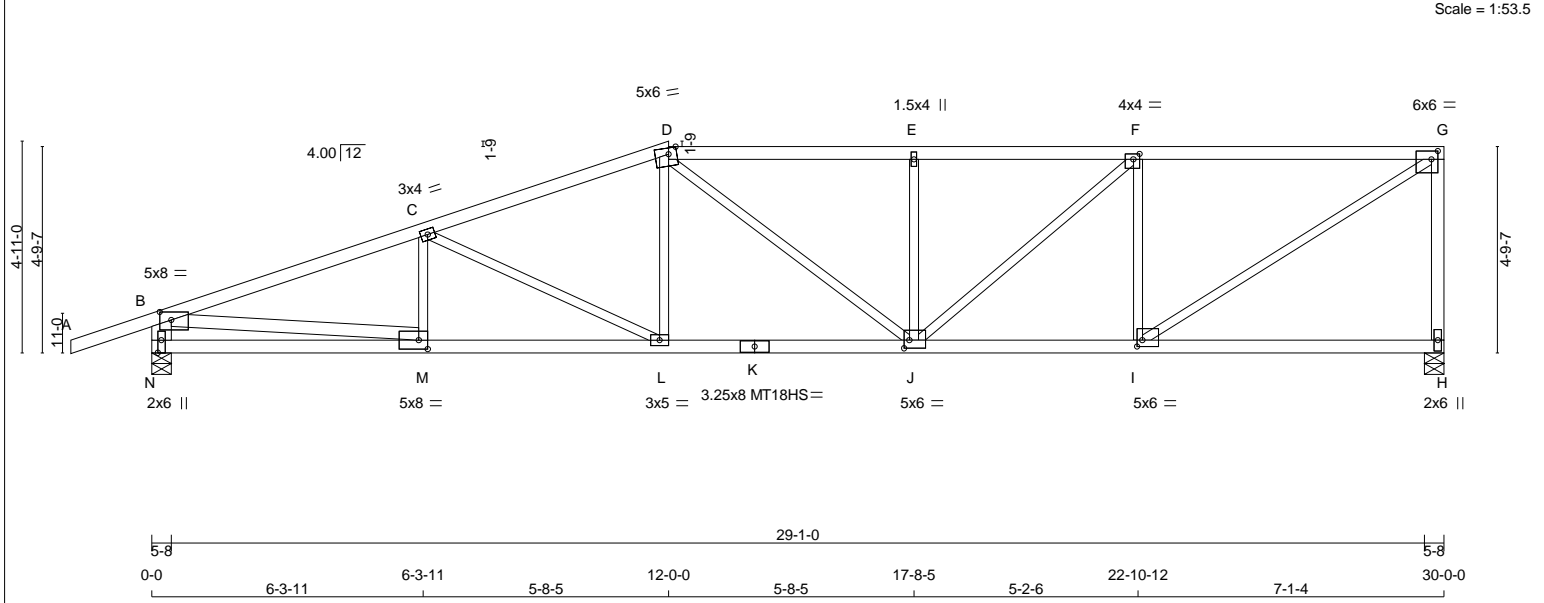


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 Mitek Industries, Inc. Fri Mar 27 13:21:00 2020 Page 1 ID:22By?JwP3U7imqHqgvnn1_zTkZ8-ZOIMq5NiUSZ0U1ZV92nuqq2UsMOZNPwMzucInAZWhrH



TOTAL WEIGHT = 118 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 2100F 1.8E	SPF
D - G	2x4	DRY 2100F 1.8E	SPF
H - G	2x4	DRY 1650F 1.5E	SPF
N - B	2x6	DRY 1650F 1.5E	SPF
N - K	2x4	DRY 1650F 1.5E	SPF
K - H	2x4	DRY 1650F 1.5E	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
B - M	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	2353	0	2353	0
N	2621	0	2621	0

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.25	3.25
C	TMVW-t	MT20	3.0	4.0		
D	TTWW-m	MT20	5.0	6.0	Edge	2.25
E	TMW+w	MT20	1.5	4.0		
F	TMVW-t	MT20	4.0	4.0	1.50	1.75
G	TMVW-t	MT20	6.0	6.0	2.25	1.75
H	BMV1+p	MT20	2.0	6.0		
I	BMVW-t	MT20	5.0	6.0	1.75	1.50
J	BMVWWW-t	MT20	5.0	6.0	2.25	1.50
K	BS-t	MT18HS	3.25	8.0		
L	BMVW-t	MT20	3.0	5.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.50
N	BMV1+p	MT20	2.0	6.0	Edge	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
H	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
N	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

LOADING

TOTAL LOAD CASES: (4)

C H O R D S		W E B S	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 42	M-C	-345 / 34
B-C	-4736 / 0	L-D	-866 / 0
C-D	-3961 / 0	L-D	0 / 494
D-E	-3881 / 0	D-J	0 / 193
E-F	-3880 / 0	J-E	-812 / 0
F-G	-2987 / 0	J-F	0 / 1180
H-G	-2300 / 0	I-F	-1763 / 0
N-B	-2567 / 0	I-G	0 / 3535
		B-M	0 / 4542
N-M	0 / 0		
M-L	0 / 4520		
L-K	0 / 3730		
K-J	0 / 3730		
J-I	0 / 2987		
I-H	0 / 0		

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/885 (0.41")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.59/1.00 (L-M:1) , WB=0.80/1.00 (G-I:1) , SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
MT18HS	586	403	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE

Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H5	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 28 of 75	H5X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:00 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-ZOIMg5NiUSZ0U1ZV92nuqq2UsMOZNPwMzuclnAzWrrH

JSI GRIP= 0.90 (M) (INPUT = 0.90)
 JSI METAL= 0.81 (M) (INPUT = 1.00)



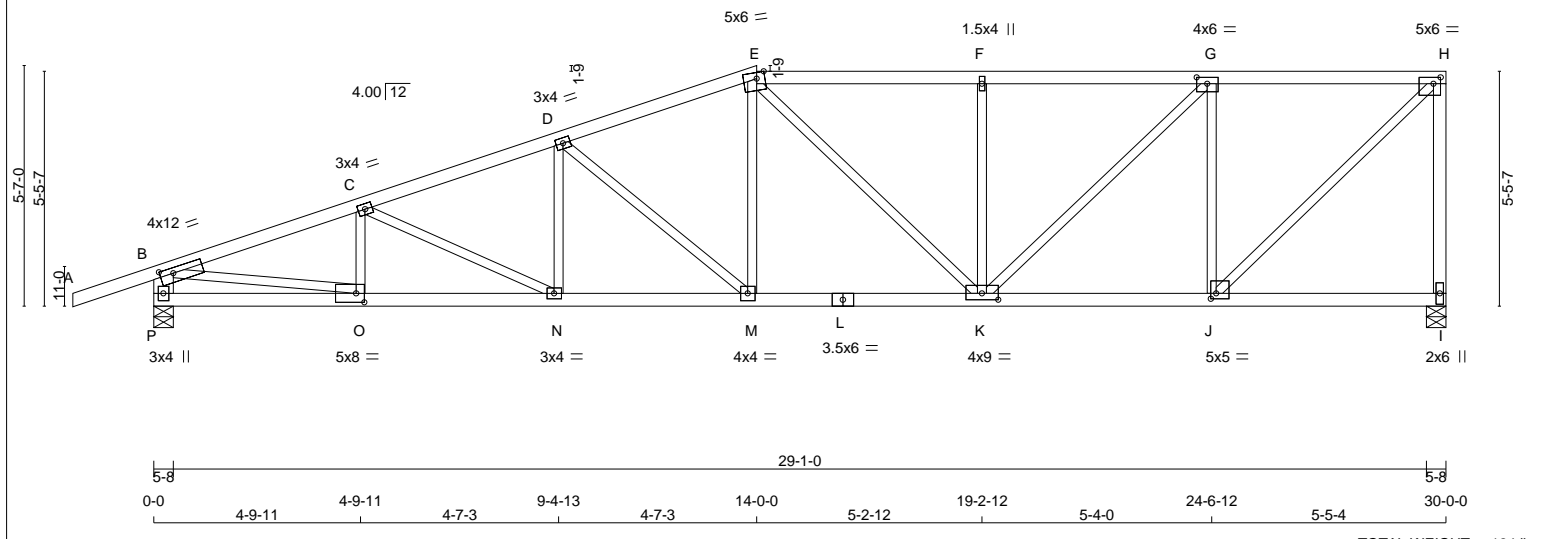
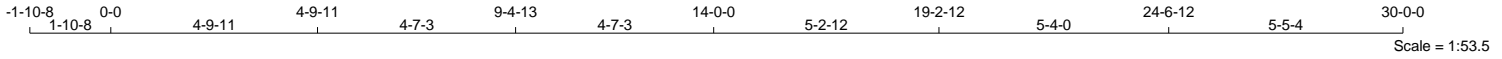
This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:01 2020 Page 1

ID:22By?JwP3U7imqHQqvn1_zTKZ8-2bJktROKFlht6B7hjl7N2beXmkw6p6VCYLJdzWhrG



TOTAL WEIGHT = 124 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY 1650F 1.5E	SPF
E - H	2x4	DRY 1650F 1.5E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
P - B	2x6	DRY 1650F 1.5E	SPF
P - L	2x4	DRY 1650F 1.5E	SPF
L - I	2x4	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	12.0	1.50	3.75
C	TMVW-t	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
E	TTWW-m	MT20	5.0	6.0	Edge	2.25
F	TMVW-w	MT20	1.5	4.0		
G	TMVW-t	MT20	4.0	6.0	1.75	3.00
H	TMVW-t	MT20	5.0	6.0	1.75	2.00
I	BMV1+P	MT20	2.0	6.0		
J	BMVW-t	MT20	5.0	5.0	1.50	1.50
K	BMVWWW-t	MT20	4.0	9.0	1.75	4.50
L	BS-t	MT20	3.5	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	5.0	8.0	2.50	2.25
P	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	2353	0	2353	0	5-8	4-2
P	2621	0	2621	0	5-8	3-0

UNFACTORED REACTIONS

JT	1ST LC CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
P	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRAC	W E B S	
	FR-TO	FROM TO					MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 42	-139.4 -139.4	0.30 (1)	10.00	0-C	-497 / 0	0.08 (1)	
B-C	-4593 / 0	-139.4 -139.4	0.53 (1)	3.38	C-N	-272 / 0	0.12 (1)	
C-D	-4348 / 0	-139.4 -139.4	0.48 (1)	3.50	N-D	0 / 204	0.05 (1)	
D-E	-3571 / 0	-139.4 -139.4	0.43 (1)	3.85	D-M	-968 / 0	0.57 (1)	
E-F	-3223 / 0	-139.4 -139.4	0.54 (1)	3.88	M-E	0 / 713	0.16 (1)	
F-G	-3222 / 0	-139.4 -139.4	0.59 (1)	3.80	E-K	-209 / 0	0.23 (1)	
G-H	-2116 / 0	-139.4 -139.4	0.53 (1)	4.58	K-F	-836 / 0	0.37 (1)	
I-H	-2314 / 0	0.0	0.0 0.91 (1)	6.33	K-G	0 / 1539	0.35 (1)	
P-B	-2572 / 0	0.0	0.0 0.10 (1)	7.23	J-G	-1901 / 0	0.85 (1)	
					J-H	0 / 2916	0.66 (1)	
P-O	0 / 0	-17.5 -17.5	0.08 (1)	10.00	B-O	0 / 4408	0.99 (1)	
O-N	0 / 4372	-17.5 -17.5	0.58 (1)	10.00				
N-M	0 / 4127	-17.5 -17.5	0.52 (1)	10.00				
M-L	0 / 3370	-17.5 -17.5	0.43 (1)	10.00				
L-K	0 / 3370	-17.5 -17.5	0.43 (1)	10.00				
K-J	0 / 2116	-17.5 -17.5	0.30 (1)	10.00				
J-I	0 / 0	-17.5 -17.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/902 (0.40")

CSI: TC=0.91/1.00 (H-I:1), BC=0.58/1.00 (N-O:1), WB=0.99/1.00 (B-O:1), SSI=0.35/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.87 (L) (INPUT = 1.00)

CONTINUE ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H6	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 30 of 75	H6X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:01 2020 Page 2
ID:22By?JwP3U7imgHQgvnn1_zTkZ8-2bJktROKFlht6B7hjl7N2beXmkw6p6VCYLJdzWhrG

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



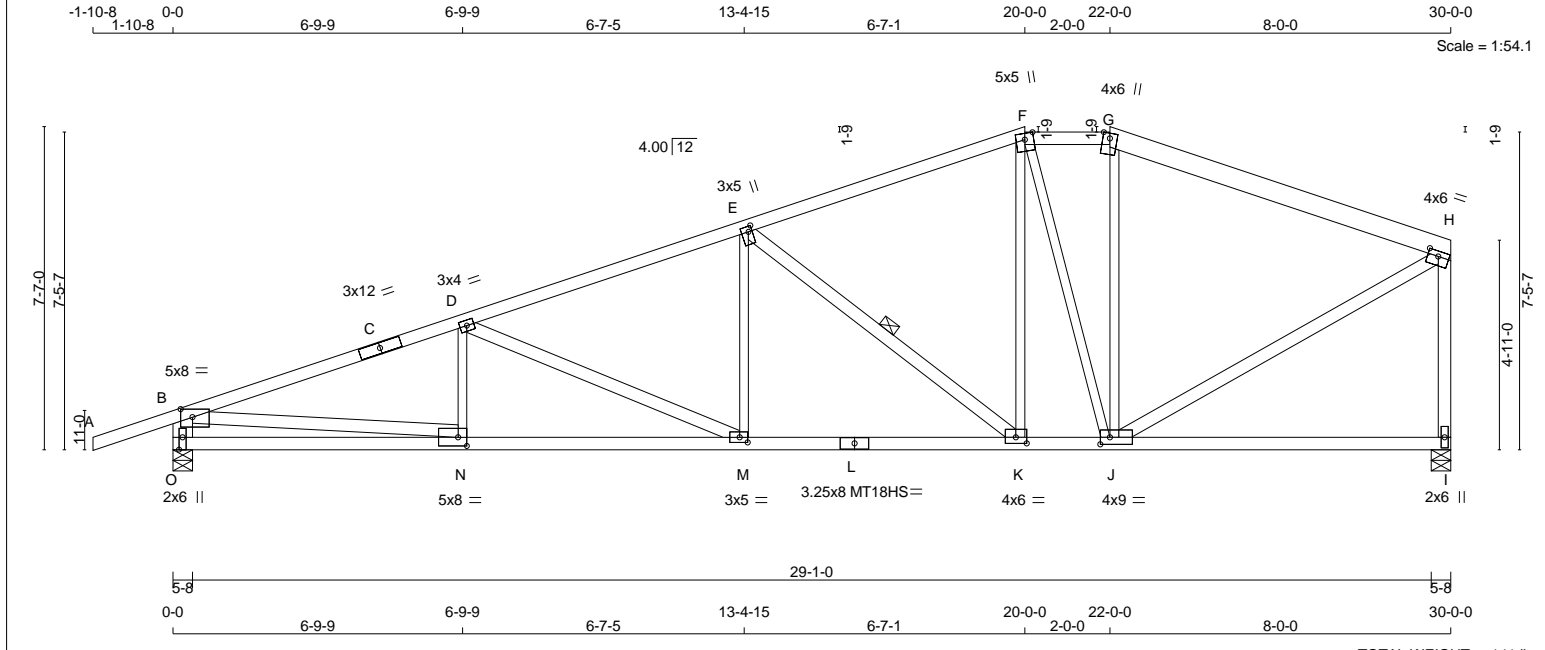
This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss-Plate-Institute.com, 781-N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITEK Industries, Inc. Fri Mar 27 13:21:03 2020 Page 1

ID:22By?JwP3U7imqHQvnn1_zTkZ8-zRUI6QamNxbLVH4rAKbStg?GZP4ainofsqPOVzWhrE



TOTAL WEIGHT = 141 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY 1650F 1.5E	SPF
C - F	2x4	DRY 1650F 1.5E	SPF
F - G	2x4	DRY 1650F 1.5E	SPF
G - H	2x6	DRY 1650F 1.5E	SPF
O - B	2x6	DRY 1650F 1.5E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
O - L	2x4	DRY 1650F 1.5E	SPF
L - I	2x4	DRY 1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
D - M	2x4	DRY 1650F 1.5E	SPF
E - K	2x4	DRY 1650F 1.5E	SPF
B - N	2x4	DRY 1650F 1.5E	SPF
J - H	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.25	3.25
C	TS-t	MT20	3.0	12.0		
D	TMVW-t	MT20	3.0	4.0		
E	TMVW+t	MT20	3.0	5.0	1.50	1.00
F	TTWW+m	MT20	5.0	5.0	Edge	
G	TTW+m	MT20	4.0	6.0	Edge	
H	TMVW-t	MT20	4.0	6.0	1.50	3.00
I	BMV1+P	MT20	2.0	6.0		
J	BMVWW-t	MT20	4.0	9.0	2.00	2.75
K	BMVWW-t	MT20	4.0	6.0	1.75	3.00
L	BS-t	MT18HS	3.25	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG UPLIFT	REQRD BRG IN-SX
O	2621	0	2621	0	0	5-8 3-0
I	2353	0	2353	0	0	5-8 4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. COMPONENT REACTIONS LIVE	PERMLIVE	WIND	DEAD	SOIL
O	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K. DBS = 18-0-0. CBF = 196 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-139.4 -139.4	0.30 (1)	N-D	-293 / 57	0.05 (1)	
B-C	-4718 / 0	-139.4 -139.4	0.89 (1)	D-M	-1028 / 0	0.56 (1)	
C-D	-4718 / 0	-139.4 -139.4	0.89 (1)	M-E	0 / 540	0.12 (1)	
D-E	-3752 / 0	-139.4 -139.4	0.68 (1)	E-K	-1741 / 0	0.42 (1)	
E-F	-2329 / 0	-139.4 -139.4	0.64 (1)	K-F	0 / 1093	0.25 (1)	
F-G	-1927 / 0	-139.4 -139.4	0.08 (1)	F-J	-885 / 0	0.98 (1)	
G-H	-2027 / 0	-139.4 -139.4	0.40 (1)	J-G	-102 / 62	0.10 (1)	
O-B	-2565 / 0	0.0 0.0	0.10 (1)	B-N	0 / 4520	0.53 (1)	
I-H	-2299 / 0	0.0 0.0	0.64 (1)	J-H	0 / 2219	0.26 (1)	
O-N	0 / 0	-17.5 -17.5	0.14 (4)				
N-M	0 / 4502	-17.5 -17.5	0.60 (1)				
M-L	0 / 3560	-17.5 -17.5	0.49 (1)				
L-K	0 / 3560	-17.5 -17.5	0.49 (1)				
K-J	0 / 2174	-17.5 -17.5	0.38 (1)				
J-I	0 / 0	-17.5 -17.5	0.21 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/936 (0.38")

CSI: TC=0.89/1.00 (B-D:1) , BC=0.60/1.00 (M-N:1) , WB=0.98/1.00 (F-J:1) , SSI=0.41/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX MIN	650 371	1747 788	1987 1873
MT20	650 371	1747 788	1987 1873
MT18HS	586 403	2455 1382	3163 3004

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H7	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 32 of 75	H7X
--------------------	------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:03 2020 Page 2
ID:22By?JwP3U7imqHQgynn1_zTkZ8-zRUI6QamNxbLVH4rAKbSTg?GZP4ainofsqPOVzWhrE

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMW-t	MT20	3.0	5.0	1.50	2.25
N	BMW-t	MT20	5.0	8.0	2.50	2.50
O	BMV1+p	MT20	2.0	6.0	Edge	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

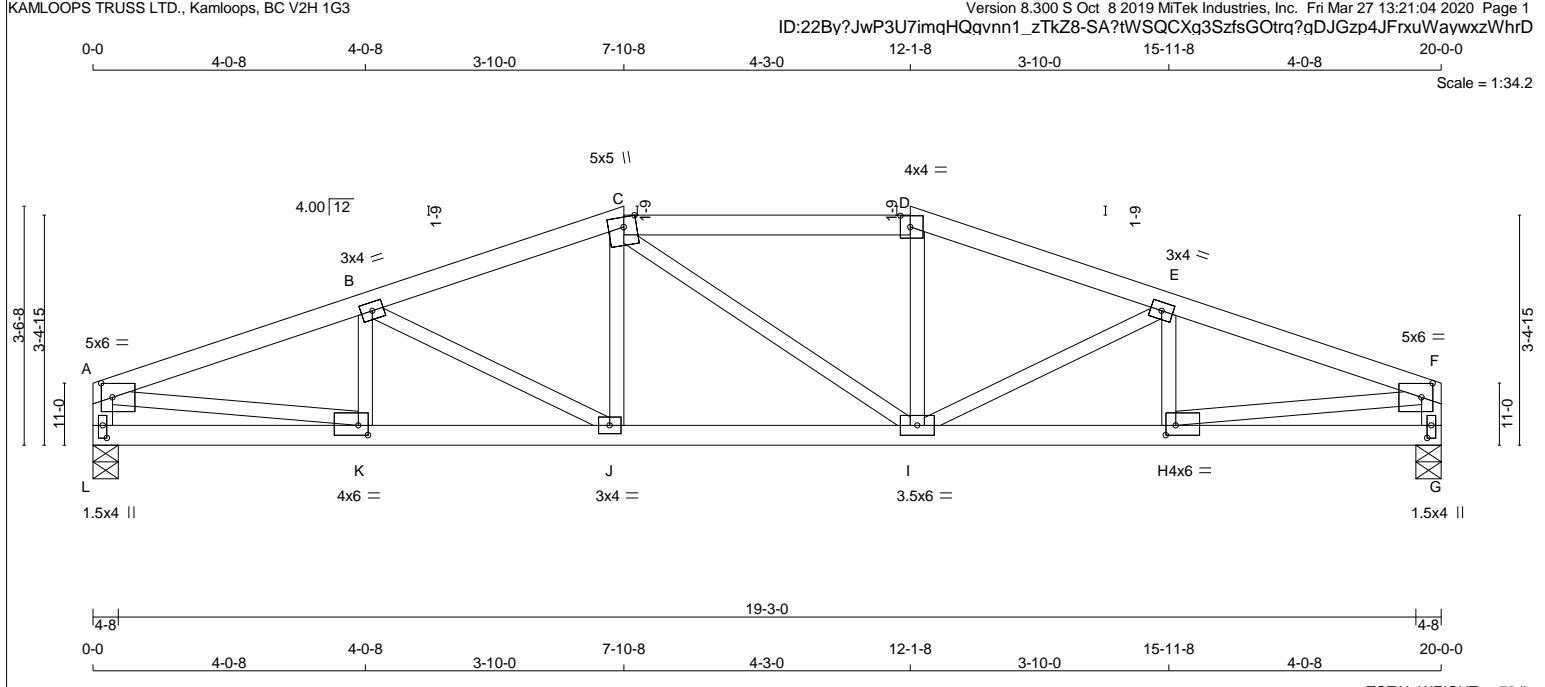
JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.80 (N) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 72 lb [M]F

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - C	2x4	DRY	1650F 1.5E	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
L - A	2x4	DRY	1650F 1.5E	SPF
G - F	2x4	DRY	1650F 1.5E	SPF
L - G	2x4	DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.50	2.00
B	TMVW-t	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	5.0	Edge	2.25
D	TTW-t	MT20	4.0	4.0	2.00	1.75
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-p	MT20	5.0	6.0	2.50	2.00
G	BMV1+p	MT20	1.5	4.0	2.25	0.75
H	BMVW-t	MT20	4.0	6.0	1.75	1.75
I	BMVW-t	MT20	3.5	6.0		
J	BMVW-t	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0	1.75	1.75
L	BMV1+p	MT20	1.5	4.0	2.25	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	IN-SX	IN-SX
L	1569	0	1569	0
G	1569	0	1569	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
L	1086	846 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
G	1086	846 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-2798 / 0	-139.4	-139.4 0.24 (1)	4.49	K-B	-361 / 0	0.06 (1)
B-C	-2502 / 0	-139.4	-139.4 0.22 (1)	4.71	B-J	-341 / 0	0.10 (1)
C-D	-2361 / 0	-139.4	-139.4 0.30 (1)	4.72	J-C	0 / 244	0.05 (1)
D-E	-2502 / 0	-139.4	-139.4 0.22 (1)	4.71	C-I	0 / 0	0.00 (1)
E-F	-2797 / 0	-139.4	-139.4 0.24 (1)	4.49	I-D	0 / 244	0.05 (1)
L-A	-1526 / 0	0.0	0.0 0.10 (1)	7.42	I-E	-340 / 0	0.10 (1)
G-F	-1526 / 0	0.0	0.0 0.10 (1)	7.42	H-E	-361 / 0	0.06 (1)
L-K	0 / 0	-17.5	-17.5 0.06 (1)	10.00	A-K	0 / 2701	0.61 (1)
K-J	0 / 2670	-17.5	-17.5 0.36 (1)	10.00	H-F	0 / 2701	0.61 (1)
J-I	0 / 2361	-17.5	-17.5 0.30 (1)	10.00			
I-H	0 / 2670	-17.5	-17.5 0.36 (1)	10.00			
H-G	0 / 0	-17.5	-17.5 0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
 CALCULATED VERT. DEFL.(LL) = L / 999 (0.11")
 ALLOWABLE DEFL.(TL)= L/360 (0.67")
 CALCULATED VERT. DEFL.(TL) = L / 999 (0.18")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.36/1.00 (J-K:1) , WB=0.61/1.00 (A-K:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

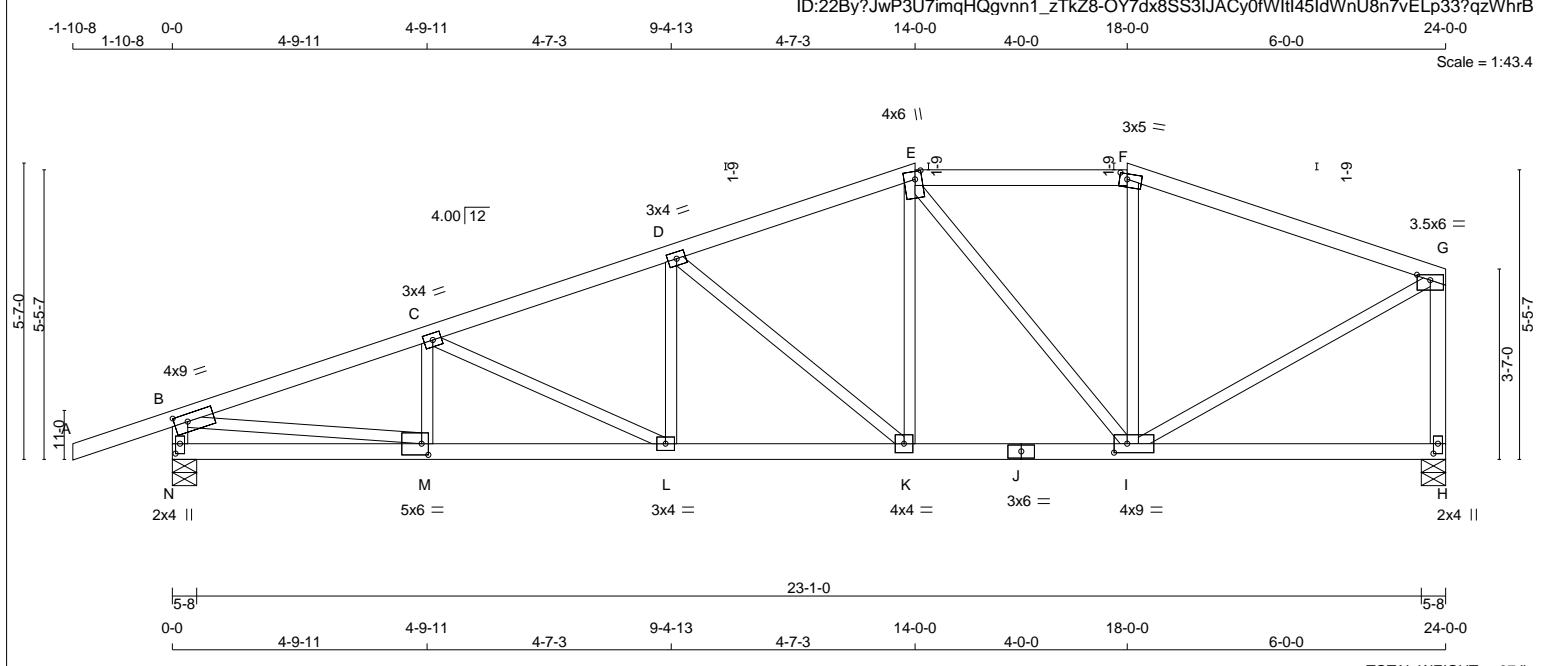
JSI GRIP= 0.90 (A) (INPUT = 0.90)
 JSI METAL= 0.64 (K) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.com and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 97 lb [M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY 1650F 1.5E	SPF
E - F	2x4	DRY 1650F 1.5E	SPF
F - G	2x4	DRY 2100F 1.8E	SPF
N - B	2x4	DRY 1650F 1.5E	SPF
H - G	2x4	DRY 1650F 1.5E	SPF
N - J	2x4	DRY 1650F 1.5E	SPF
J - H	2x4	DRY 1650F 1.5E	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
E	TTWW+m	MT20	4.0	6.0	1.75	1.50
F	TTW-m	MT20	3.0	5.0	1.25	1.75
G	TMVW-p	MT20	3.5	6.0	1.25	3.00
H	BMV1+p	MT20	2.0	4.0	2.25	1.00
I	BMVWW-t	MT20	4.0	9.0	2.00	3.00
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.50	1.50
N	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
N	2150	0	2150	0	5-8	3-8
H	1883	0	1883	0	5-8	2-13

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
N	1485	1178 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0
H	1303	1015 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED CSI (LC)	MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)
A-B	0 / 42	-139.4	-139.4	0.30 (1)	M-C	-364 / 6	0.06 (1)
B-C	-3531 / 0	-139.4	-139.4	0.39 (1)	C-L	-461 / 0	0.20 (1)
C-D	-3109 / 0	-139.4	-139.4	0.30 (1)	L-D	0 / 292	0.07 (1)
D-E	-2244 / 0	-139.4	-139.4	0.30 (1)	D-K	-1083 / 0	0.63 (1)
E-F	-1618 / 0	-139.4	-139.4	0.25 (1)	K-E	0 / 759	0.17 (1)
F-G	-1697 / 0	-139.4	-139.4	0.44 (1)	E-I	-787 / 0	0.63 (1)
N-B	-2104 / 0	0.0	0.0	0.13 (1)	I-F	-175 / 45	0.08 (1)
H-G	-1839 / 0	0.0	0.0	0.27 (1)	I-G	0 / 1849	0.42 (1)
					B-M	0 / 3394	0.76 (1)
N-M	0 / 0	-17.5	-17.5	0.07 (4)			
M-L	0 / 3366	-17.5	-17.5	0.45 (1)			
L-K	0 / 2951	-17.5	-17.5	0.37 (1)			
K-J	0 / 2105	-17.5	-17.5	0.30 (1)			
J-I	0 / 2105	-17.5	-17.5	0.30 (1)			
I-H	0 / 0	-17.5	-17.5	0.11 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.44/1.00 (F-G:1) , BC=0.45/1.00 (L-M:1) , WB=0.76/1.00 (B-M:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

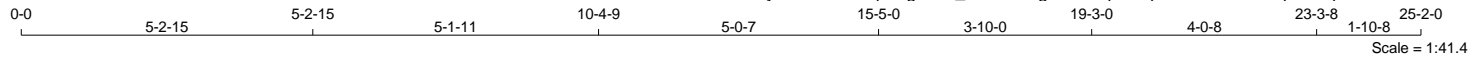
JSI GRIP= 0.90 (N) (INPUT = 0.90)
 JSI METAL= 0.77 (M) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - A	2x4 DRY	1650F 1.5E	SPF
A - D	2x4 DRY	1650F 1.5E	SPF
D - G	2x4 DRY	1650F 1.5E	SPF
H - F	2x6 DRY	1650F 1.5E	SPF
M - J	2x4 DRY	1650F 1.5E	SPF
J - H	2x4 DRY	1650F 1.5E	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
E - H	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	1.75
B	TMVW-t	MT20	4.0	4.0	1.75	1.75
C	TMW+w	MT20	1.5	4.0		
D	TTWW-m	MT20	5.0	6.0	Edge	2.25
E	TMVW-t	MT20	5.0	5.0	2.00	2.00
F	TMV+P	MT20	1.5	4.0		
H	BMVW1-t	MT20	4.0	9.0	2.00	3.75
I	BMVW1-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWVW-t	MT20	3.0	8.0	1.50	3.25
L	BMVW-t	MT20	4.0	6.0	1.50	1.50
M	BMV1+P	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
M	1827	0	5-8	2-11
H	2095	0	5-8	2-4

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1265	985 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
H	1446	1148 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO			
M-A	-1787 / 0	0.0	0.0 0.23 (1)	A-L	0 / 2915	0.66 (1)	
A-B	-2506 / 0	-139.4	-139.4 0.50 (1)	L-B	-1386 / 0	0.28 (1)	
B-C	-3473 / 0	-139.4	-139.4 0.55 (1)	B-K	0 / 1131	0.25 (1)	
C-D	-3474 / 0	-139.4	-139.4 0.49 (1)	K-C	-808 / 0	0.16 (1)	
D-E	-3187 / 0	-139.4	-139.4 0.31 (1)	K-D	0 / 541	0.12 (1)	
E-F	0 / 17	-139.4	-139.4 0.24 (1)	I-D	0 / 211	0.05 (1)	
F-G	0 / 42	-139.4	-139.4 0.30 (1)	I-E	-158 / 58	0.05 (1)	
H-F	-494 / 0	0.0	0.0 0.02 (1)	E-H	-3519 / 0	0.57 (1)	
M-L	0 / 0	-17.5	-17.5 0.10 (4)				
L-K	0 / 2506	-17.5	-17.5 0.34 (1)				
K-J	0 / 3016	-17.5	-17.5 0.43 (1)				
J-I	0 / 3016	-17.5	-17.5 0.43 (1)				
I-H	0 / 3163	-17.5	-17.5 0.48 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 1919
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/989 (0.28")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.48/1.00 (H-I:1) , WB=0.66/1.00 (A-L:1) , SSI=0.34/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PL)	SECTION (PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

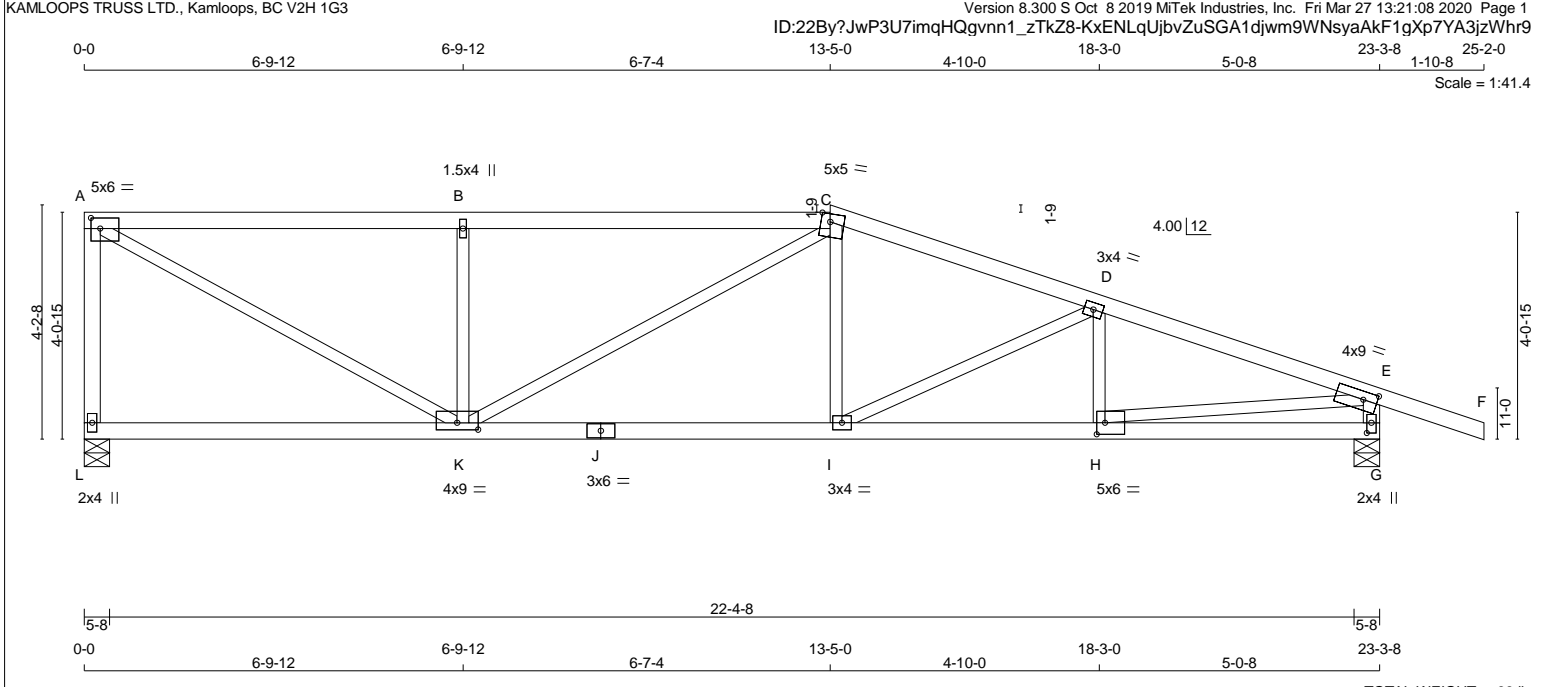
JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.92 (E) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 88 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 L - A 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 2100F 1.8E SPF
 C - F 2x4 DRY 1650F 1.5E SPF
 G - E 2x4 DRY 1650F 1.5E SPF
 L - J 2x4 DRY 1650F 1.5E SPF
 J - G 2x4 DRY 1650F 1.5E SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.00
B	TMW+w	MT20	1.5	4.0		
C	TTWW-m	MT20	5.0	5.0	1.75	2.00
D	TMVW-t	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	9.0	1.75	3.00
G	BMV1+p	MT20	2.0	4.0	2.25	1.00
H	BMVW-t	MT20	5.0	6.0	2.50	1.75
I	BMVW-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	9.0	1.50	4.50
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
L	1827	0	1827	0
G	2095	0	2095	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
L	1265	985 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	1446	1148 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)
L-A	-1776 / 0	0.0	0.0 0.33 (1)	A-K	0 / 2906	0.65 (1)	
A-B	-2540 / 0	-139.4	-139.4 0.83 (1)	K-B	-1182 / 0	0.30 (1)	
B-C	-2541 / 0	-139.4	-139.4 0.82 (1)	K-C	-200 / 0	0.24 (1)	
C-D	-2883 / 0	-139.4	-139.4 0.52 (1)	I-C	0 / 389	0.09 (1)	
D-E	-3446 / 0	-139.4	-139.4 0.56 (1)	I-D	-631 / 0	0.29 (1)	
E-F	0 / 42	-139.4	-139.4 0.30 (1)	H-D	-337 / 13	0.05 (1)	
G-E	-2046 / 0	0.0	0.0 0.13 (1)	H-E	0 / 3315	0.75 (1)	
L-K	0 / 0	-17.5	-17.5 0.17 (4)				10.00
K-J	0 / 2713	-17.5	-17.5 0.38 (1)				10.00
J-I	0 / 2713	-17.5	-17.5 0.38 (1)				10.00
I-H	0 / 3291	-17.5	-17.5 0.44 (1)				10.00
H-G	0 / 0	-17.5	-17.5 0.08 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL)= L/360 (0.78")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.83/1.00 (A-B:1), BC=0.44/1.00 (H-I:1), WB=0.75/1.00 (E-H:1), SSI=0.46/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
 JSI METAL= 0.85 (J) (INPUT = 1.00)

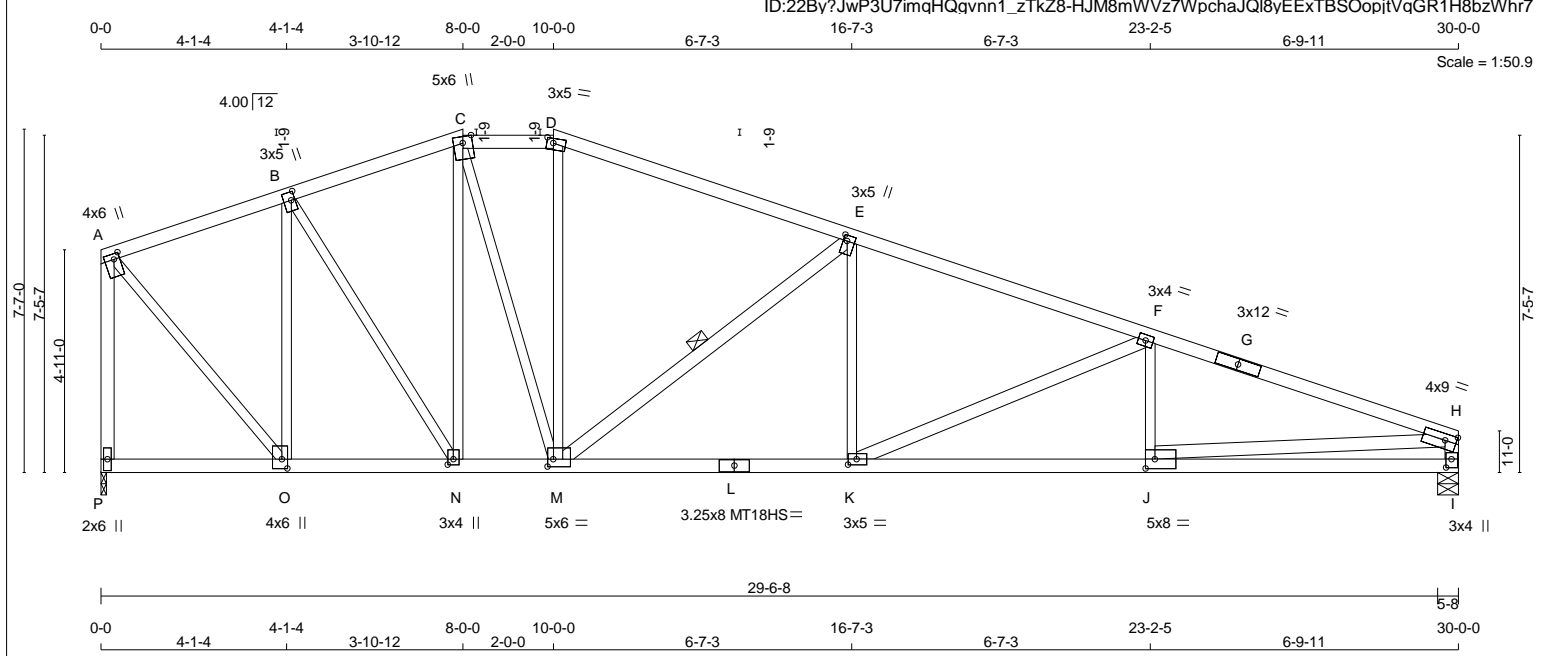


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:10 2020 Page 1



TOTAL WEIGHT = 138 lb [M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY 1650F 1.5E	SPF
C - D	2x4	DRY 1650F 1.5E	SPF
D - G	2x4	DRY 1650F 1.5E	SPF
G - H	2x4	DRY 1650F 1.5E	SPF
P - A	2x4	DRY 1650F 1.5E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
P - L	2x4	DRY 1650F 1.5E	SPF
L - I	2x4	DRY 1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
M - E	2x4	DRY 1650F 1.5E	SPF
K - F	2x4	DRY 1650F 1.5E	SPF
J - H	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	6.0	1.50	1.50
B	TMVW+t	MT20	3.0	5.0	2.25	1.00
C	TTW+m	MT20	5.0	6.0	Edge	
D	TTW-m	MT20	3.0	5.0	1.25	1.75
E	TMVW+t	MT20	3.0	5.0	1.50	1.00
F	TMVW-t	MT20	3.0	4.0		
G	TS-t	MT20	3.0	12.0		
H	TMVW-t	MT20	4.0	9.0	1.75	3.00
I	BMV1+tp	MT20	3.0	4.0	2.25	1.50
J	BMVW-t	MT20	5.0	8.0	2.50	2.50
K	BMVW-t	MT20	3.0	5.0	1.50	2.25
L	BS-t	MT18HS	3.25	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	DOWN	UP	IN-SX	REQRD BRG
P	2353	0	2353	0	0	1-8	
I	2353	0	2353	0	0	5-8	4-2

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
P	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 18-0-0. CBF = 196 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	-1499 / 0	-139.4 -139.4	0.25 (1)	5.69	O-B	-1544 / 0	0.98 (1)
B-C	-2018 / 0	-139.4 -139.4	0.25 (1)	5.08	B-N	0 / 864	0.19 (1)
C-D	-2189 / 0	-139.4 -139.4	0.08 (1)	5.11	N-C	-685 / 0	0.71 (1)
D-E	-2332 / 0	-139.4 -139.4	0.64 (1)	4.24	C-M	0 / 986	0.22 (1)
E-F	-3752 / 0	-139.4 -139.4	0.68 (1)	3.50	M-D	0 / 201	0.05 (1)
F-G	-4718 / 0	-139.4 -139.4	0.89 (1)	2.87	M-E	-1738 / 0	0.42 (1)
G-H	-4718 / 0	-139.4 -139.4	0.89 (1)	2.87	K-E	0 / 536	0.12 (1)
P-A	-2323 / 0	0.0 0.0	0.72 (1)	6.32	K-F	-1027 / 0	0.56 (1)
I-H	-2298 / 0	0.0 0.0	0.15 (1)	6.35	J-F	-292 / 58	0.05 (1)
P-O	0 / 0	-17.5 -17.5	0.06 (4)	10.00	A-O	0 / 2170	0.49 (1)
O-N	0 / 1441	-17.5 -17.5	0.20 (1)	10.00	J-H	0 / 4520	0.53 (1)
N-M	0 / 1914	-17.5 -17.5	0.25 (1)	10.00			
M-L	0 / 3560	-17.5 -17.5	0.48 (1)	10.00			
L-K	0 / 3560	-17.5 -17.5	0.48 (1)	10.00			
K-J	0 / 4502	-17.5 -17.5	0.59 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/922 (0.39")

CSI: TC=0.89/1.00 (F-H:1), BC=0.59/1.00 (J-K:1), WB=0.98/1.00 (B-O:1), SSI=0.41/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
MT18HS	586	403	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.80 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H12	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 38 of 75	H12X
--------------------	-------------------	---------------	----------	--------------------------------	----------------------	-------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:10 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-HJM8mWVz7WpchaJQl8yEEExTBSOopjtVqGR1H8bzWHR7

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMWW+t	MT20	5.0	6.0	2.00	1.50
N	BMWW+t	MT20	3.0	4.0	1.50	1.50
O	BMWW+t	MT20	4.0	6.0	2.50	1.50
P	BMV1+p	MT20	2.0	6.0		

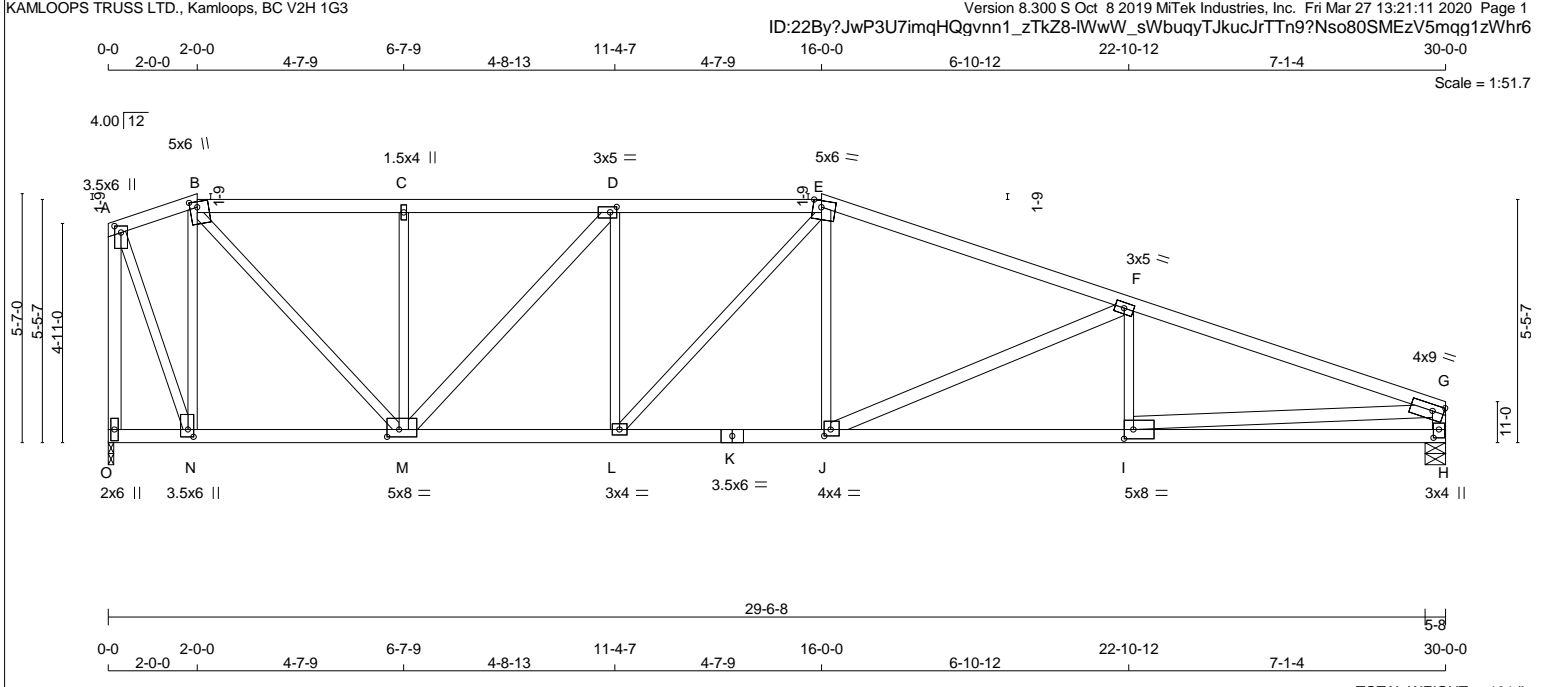
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 131 lb [M]F

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - B	2x4	DRY	1650F 1.5E	SPF
B - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
O - A	2x4	DRY	1650F 1.5E	SPF
H - G	2x4	DRY	1650F 1.5E	SPF
O - K	2x4	DRY	1650F 1.5E	SPF
K - H	2x4	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - D	2x4	DRY	1650F 1.5E	SPF
J - F	2x4	DRY	1650F 1.5E	SPF
I - G	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
O	2353	0	2353	0
H	2353	0	2353	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
O	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
H	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-843 / 0	-139.4	-139.4 0.06 (1)	6.25	N-B	-1894 / 0	0.89 (1)
B-C	-2414 / 0	-139.4	-139.4 0.30 (1)	4.69	M-B	0 / 2364	0.53 (1)
C-D	-2413 / 0	-139.4	-139.4 0.29 (1)	4.69	M-C	-743 / 0	0.33 (1)
D-E	-3282 / 0	-139.4	-139.4 0.33 (1)	4.11	M-D	-1283 / 0	0.63 (1)
E-F	-3595 / 0	-139.4	-139.4 0.67 (1)	3.80	L-D	0 / 181	0.04 (4)
F-G	-4764 / 0	-139.4	-139.4 0.79 (1)	3.25	L-E	-140 / 0	0.13 (1)
O-A	-2347 / 0	0.0	0.0 0.72 (1)	6.30	J-E	0 / 623	0.14 (1)
H-G	-2297 / 0	0.0	0.0 0.15 (1)	6.36	J-F	-1274 / 0	0.75 (1)
					I-F	-270 / 71	0.05 (1)
O-N	0 / 0	-17.5	-17.5 0.05 (4)	10.00	A-N	0 / 2123	0.48 (1)
N-M	0 / 844	-17.5	-17.5 0.14 (1)	10.00	I-G	0 / 4570	0.53 (1)
M-L	0 / 3280	-17.5	-17.5 0.42 (1)	10.00			
L-K	0 / 3374	-17.5	-17.5 0.45 (1)	10.00			
K-J	0 / 3374	-17.5	-17.5 0.45 (1)	10.00			
J-I	0 / 4553	-17.5	-17.5 0.59 (1)	10.00			
I-H	0 / 0	-17.5	-17.5 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/919 (0.39")

CSI: TC=0.79/1.00 (F-G:1), BC=0.59/1.00 (I-J:1), WB=0.89/1.00 (B-N:1), SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
 JSI METAL= 0.83 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	3.5	6.0	1.75	1.75
B	TTWW+m	MT20	5.0	6.0	1.50	2.00
C	TMW+w	MT20	1.5	4.0		
D	TMWW-t	MT20	3.0	5.0	1.50	1.75
E	TTWW-m	MT20	5.0	6.0	Edge	2.25
F	TMWW-t	MT20	3.0	5.0		
G	TMVW-t	MT20	4.0	9.0	1.75	3.00
H	BMV1+p	MT20	3.0	4.0	2.25	1.50
I	BMWW-t	MT20	5.0	8.0	2.50	2.50
J	BMWW-t	MT20	4.0	4.0	1.75	1.75
K	BS-t	MT20	3.5	6.0		
L	BMWW-t	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	8.0	2.00	3.25



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME H13	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 40 of 75
--------------------	-------------------	---------------	----------	--------------------------------	----------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:11 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-IWwW_sWbuqyTJkucJrTTn9?Nso80SMEzV5mqg1zWhr6

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWw+t	MT20	3.5	6.0	2.00	1.50
O	BMV1+p	MT20	2.0	6.0		

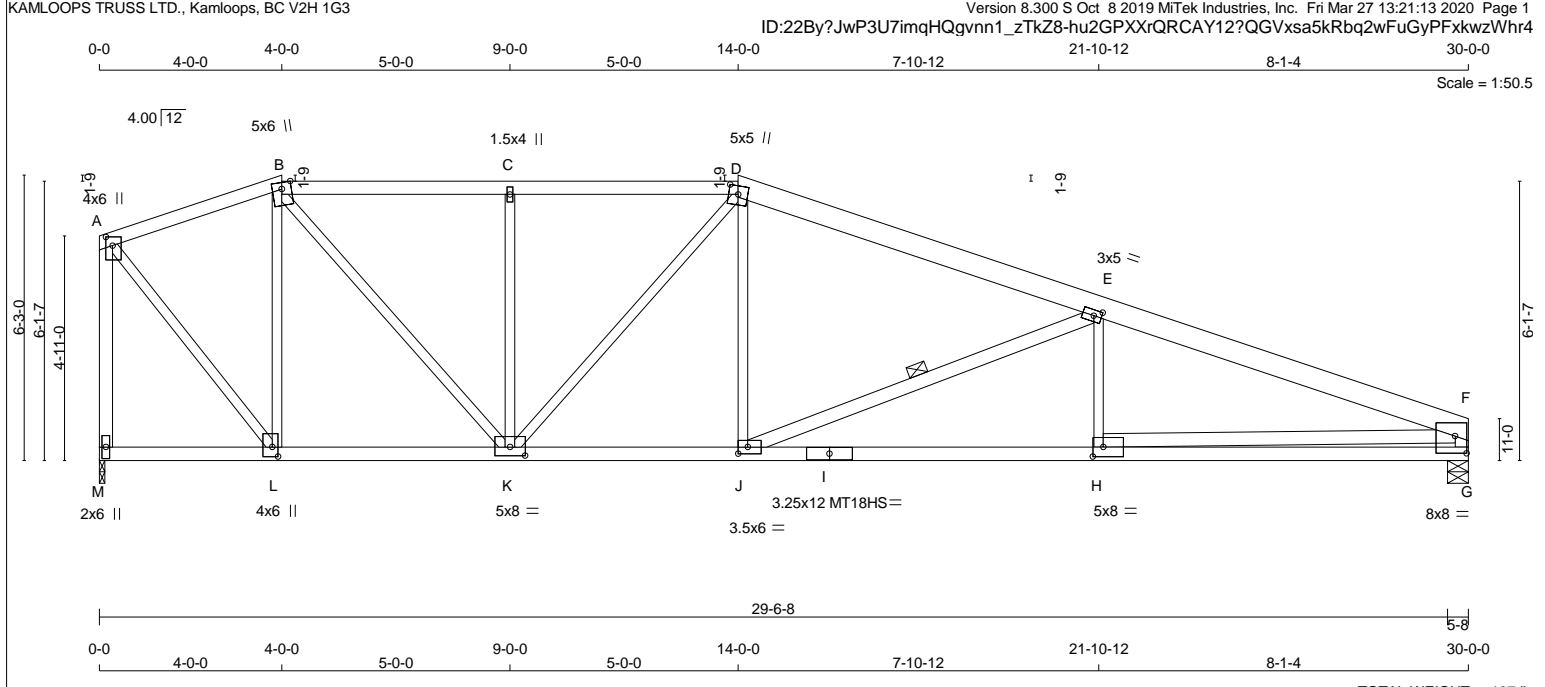
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY 1650F 1.5E	SPF
B - D	2x4	DRY 1650F 1.5E	SPF
D - F	2x6	DRY 1650F 1.5E	SPF
M - A	2x4	DRY 1650F 1.5E	SPF
G - F	2x4	DRY 1650F 1.5E	SPF
M - I	2x4	DRY 1650F 1.5E	SPF
I - G	2x4	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 J - E 2x4 DRY 1650F 1.5E SPF
 H - F 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	1.75
B	TTWW+m	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	1.5	4.0		
D	TTWW+m	MT20	5.0	5.0	2.25	2.50
E	TMVW-t	MT20	3.0	5.0	1.50	2.00
F						
G	TMBMWV1*-i	MT20	8.0	8.0	4.50	3.00
H	BMVW-t	MT20	5.0	8.0	2.50	2.75
I	BS-t	MT18HS	3.25	12.0		
J	BMVW-t	MT20	3.5	6.0	1.75	2.50
K	BMVW-t	MT20	5.0	8.0	2.25	4.00
L	BMVW-t	MT20	4.0	6.0	2.50	1.50
M	BMV1+p	MT20	2.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
M	2353 0	2353 0	0 0	1-8
G	2353 0	2353 0	0 0	5-8

UNFACTORED REACTIONS

JT	1ST LC CASE	MAX./MIN. COMPONENT REACTIONS
M	COMBINED SNOW	LIVE PERMLIVE WIND DEAD SOIL
M	1629 1269 / 0	0 / 0 0 / 0 0 / 0 360 / 0 0 / 0
G	1629 1269 / 0	0 / 0 0 / 0 0 / 0 360 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J. DBS = 18-0-0 . CBF = 194 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/922 (0.39")

CSI: TC=0.72/1.00 (A-M:1) , BC=0.62/1.00 (H-J:1) , WB=0.94/1.00 (B-L:1) , SS=0.39/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873
MT18HS	586	403	2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.90 (H) (INPUT = 0.90)
 JSI METAL= 0.83 (I) (INPUT = 1.00)



CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	-1432 / 0	-139.4	-139.4	0.25 (1)	5.78	L-B	-1522 / 0
B-C	-2618 / 0	-139.4	-139.4	0.40 (1)	4.41	B-K	0 / 1892
C-D	-2617 / 0	-139.4	-139.4	0.40 (1)	4.41	K-C	-903 / 0
D-E	-3230 / 0	-139.4	-139.4	0.36 (1)	4.91	K-D	-616 / 0
E-F	-4834 / 0	-139.4	-139.4	0.42 (1)	4.13	J-D	0 / 783
M-A	-2330 / 0	0.0	0.0	0.72 (1)	6.32	J-E	-1721 / 0
G-F	-2291 / 0	0.0	0.0	0.15 (1)	6.36	H-E	-153 / 115
M-L	0 / 0	-17.5	-17.5	0.07 (4)	10.00	A-L	0 / 2116
L-K	0 / 1392	-17.5	-17.5	0.21 (1)	10.00	H-F	0 / 4630
K-J	0 / 3017	-17.5	-17.5	0.43 (1)	10.00		
J-I	0 / 4621	-17.5	-17.5	0.62 (1)	10.00		
I-H	0 / 4621	-17.5	-17.5	0.62 (1)	10.00		
H-G	0 / 0	-17.5	-17.5	0.22 (4)	10.00		

CONTINUED ON PAGE 2

JOB NAME 200656	TRUSS NAME H14	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const. TRUSS DESC.	DRWG NO. 42 of 75
--------------------	-------------------	---------------	----------	---	----------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:13 2020 Page 2
ID:22By?JwP3U7imqHQgynn1_zTkZ8-hu2GPPXrQRcAY12?QGvXsa5kRbq2wFuGyPFxkwzWHR4

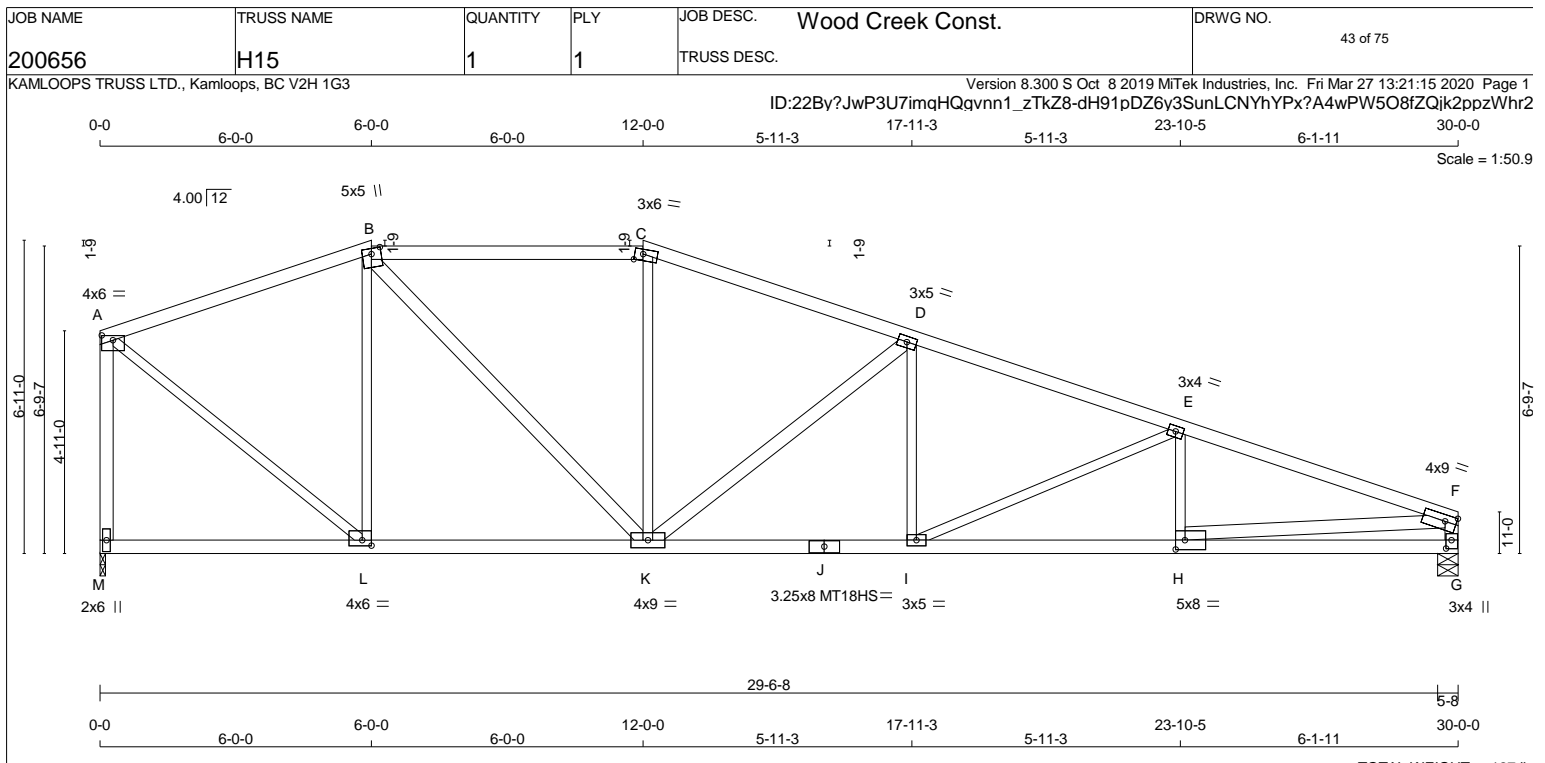
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use onlywith MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
 TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information
 available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY 2100F 1.8E	SPF
B - C	2x4	DRY 2100F 1.8E	SPF
C - F	2x4	DRY 1650F 1.5E	SPF
M - A	2x4	DRY 1650F 1.5E	SPF
G - F	2x4	DRY 1650F 1.5E	SPF
M - J	2x4	DRY 1650F 1.5E	SPF
J - G	2x4	DRY 1650F 1.5E	SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
B - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	1650F 1.5E	SPF
H - F	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.25	3.00
B	TTWW+m	MT20	5.0	5.0	1.50	2.50
C	TTW-m	MT20	3.0	6.0	Edge	2.25
D	TMVW-t	MT20	3.0	5.0		
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	9.0	1.75	3.00
G	BMV1+p	MT20	3.0	4.0	2.25	1.50
H	BMVW-t	MT20	5.0	8.0	2.50	2.50
I	BMVW-t	MT20	3.0	5.0		
J	BS-t	MT18HS	3.25	8.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	6.0	1.50	2.50
M	BMV1+p	MT20	2.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
M	2353 0	2353 0	0 0	1-8
G	2353 0	2353 0	0 0	5-8 4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. COMPONENT LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
G	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED (LC)	MEMB.	
FR-TO		FROM TO	LENGTH	FR-TO				
A-B	-1799 / 0	-139.4 -139.4	0.44 (1)	5.38	L-B	-1222 / 0	0.99 (1)	
B-C	-2605 / 0	-139.4 -139.4	0.46 (1)	4.64	B-K	0 / 1282	0.15 (1)	
C-D	-2764 / 0	-139.4 -139.4	0.53 (1)	4.15	K-C	0 / 107	0.04 (4)	
D-E	-3973 / 0	-139.4 -139.4	0.56 (1)	3.57	K-D	-1488 / 0	0.88 (1)	
E-F	-4707 / 0	-139.4 -139.4	0.72 (1)	3.12	I-D	0 / 431	0.10 (1)	
M-A	-2312 / 0	0.0	0.0	0.71 (1)	6.33	I-E	-787 / 0	0.61 (1)
G-F	-2301 / 0	0.0	0.0	0.15 (1)	6.35	H-E	-350 / 34	0.06 (1)
						A-L	0 / 2175	0.49 (1)
						H-F	0 / 4511	0.53 (1)
M-L	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
L-K	0 / 1732	-17.5	-17.5	0.27 (1)	10.00			
K-J	0 / 3770	-17.5	-17.5	0.49 (1)	10.00			
J-I	0 / 3770	-17.5	-17.5	0.49 (1)	10.00			
I-H	0 / 4488	-17.5	-17.5	0.58 (1)	10.00			
H-G	0 / 0	-17.5	-17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/970 (0.37")

CSI: TC=0.72/1.00 (E-F:1), BC=0.58/1.00 (H-I:1), WB=0.99/1.00 (B-L:1), SS=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
MT18HS	586	403	2455
	MIN	MAX	MIN
	1787	1873	3163
	3004		

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
 JSI METAL= 0.80 (H) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

JOB NAME 200656	TRUSS NAME H15	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 44 of 75
--------------------	-------------------	---------------	----------	--------------------------------	----------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:15 2020 Page 2

ID:22By?JwP3U7imqHQvnn1_zTkZ8-dH91pDZ6y3SunLCNYhYPx?A4wPW5O8fZQjk2ppzWhr2

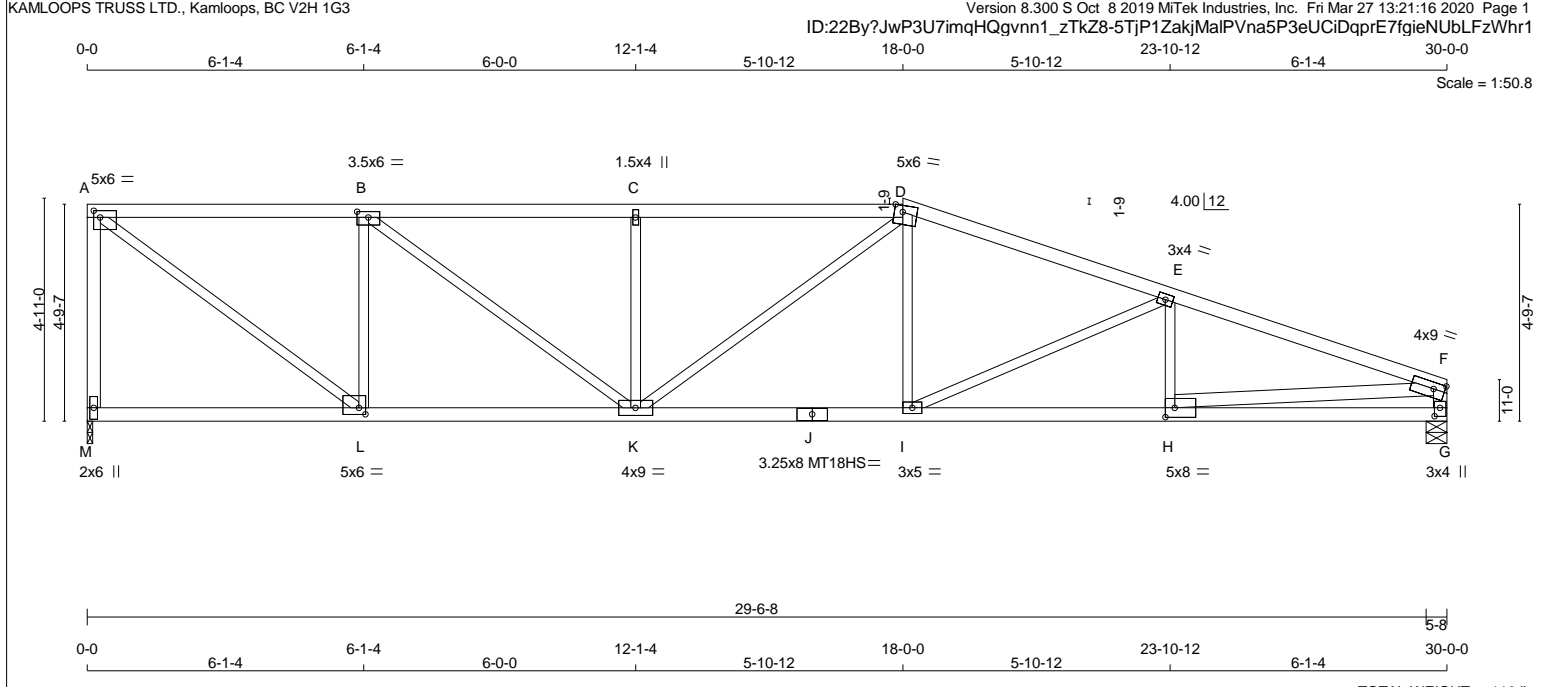
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from [Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314](http://TrussPlateInstitute.com).



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - A	2x4 DRY	1650F 1.5E	SPF
A - D	2x4 DRY	1650F 1.5E	SPF
D - F	2x4 DRY	2100F 1.8E	SPF
G - F	2x4 DRY	1650F 1.5E	SPF
M - J	2x4 DRY	1650F 1.5E	SPF
J - G	2x4 DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT H - F	2x3 DRY	No.2 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t MT20	5.0	6.0	1.75	1.75
B	TMVW-t MT20	3.5	6.0	1.50	3.00
C	TMW+w MT20	1.5	4.0		
D	TTWW-m MT20	5.0	6.0	Edge	2.25
E	TMVW-t MT20	3.0	4.0		
F	TMVW-t MT20	4.0	9.0	1.75	3.00
G	BMV1+P MT20	3.0	4.0	2.25	1.50
H	BMVW-t MT20	5.0	8.0	2.50	2.50
I	BMVW-t MT20	3.0	5.0		
J	BS-t MT18HS	3.25	8.0		
K	BMVW-t MT20	4.0	9.0		
L	BMVW-t MT20	5.0	6.0	1.75	1.75
M	BMV1+P MT20	2.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2353	2353	0	1-8
JT HORZ	0	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
G	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
M-A	-2308 / 0	0.0	0.0 0.63 (1)	A-L	0 / 3315	0.75 (1)	
A-B	-2670 / 0	-139.4	-139.4 0.73 (1)	L-B	-1845 / 0	0.62 (1)	
B-C	-3888 / 0	-139.4	-139.4 0.84 (1)	B-K	0 / 1522	0.34 (1)	
C-D	-3889 / 0	-139.4	-139.4 0.77 (1)	K-C	-941 / 0	0.32 (1)	
D-E	-3961 / 0	-139.4	-139.4 0.71 (1)	K-D	0 / 199	0.04 (1)	
E-F	-4742 / 0	-139.4	-139.4 0.81 (1)	I-D	0 / 478	0.11 (1)	
G-F	-2300 / 0	0.0	0.0 0.15 (1)	I-E	-863 / 0	0.63 (1)	
				H-E	-362 / 30	0.06 (1)	
M-L	0 / 0	-17.5	-17.5 0.13 (4)	H-F	0 / 4549	0.53 (1)	
L-K	0 / 2670	-17.5	-17.5 0.37 (1)				
K-J	0 / 3730	-17.5	-17.5 0.50 (1)				
J-I	0 / 3730	-17.5	-17.5 0.50 (1)				
I-H	0 / 4526	-17.5	-17.5 0.59 (1)				
H-G	0 / 0	-17.5	-17.5 0.11 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/869 (0.41")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.59/1.00 (H-I:1) , WB=0.75/1.00 (A-L:1) , SS=0.40/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873
MT18HS	586	403	2455 1382 3163 3004

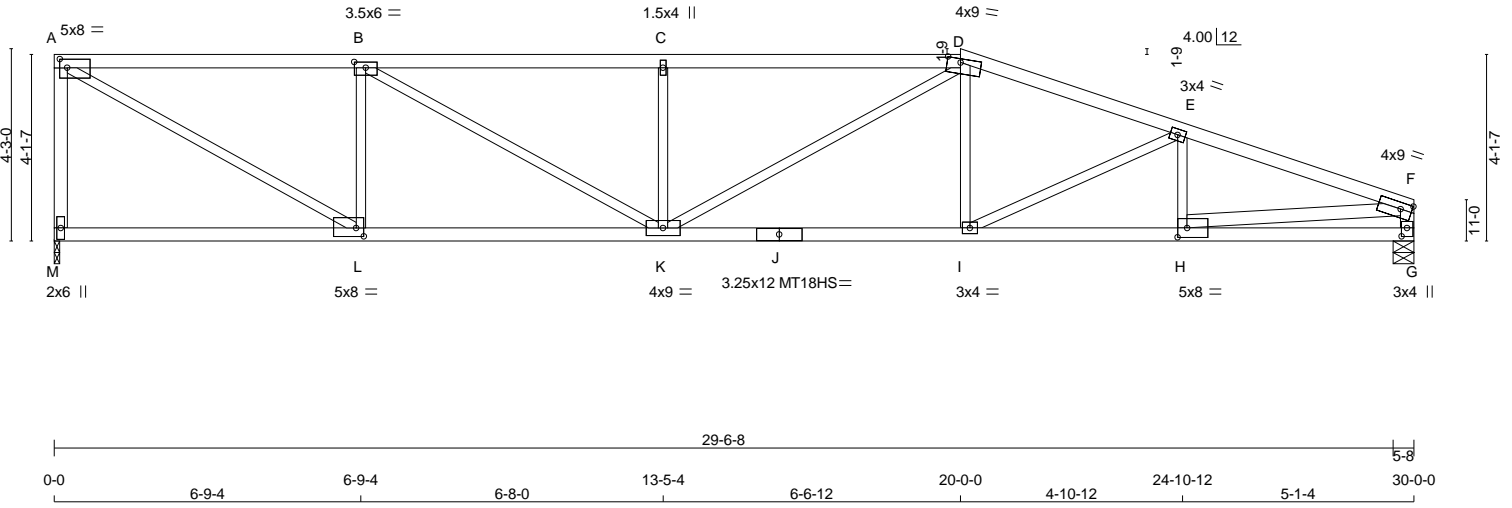
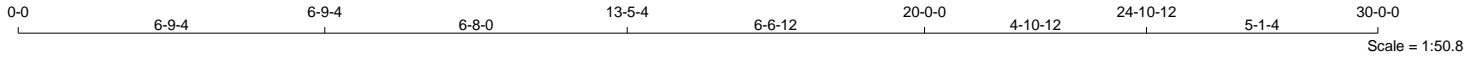
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
 JSI METAL= 0.81 (H) (INPUT = 1.00)

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:18 2020 Page 1

ID:22By?JwP3U7imqHQvnn1_zTkZ8-2sr9SFb_E_qTeoxyDp56ZdoYJcXrbX4?6hziQ7zWhr?



TOTAL WEIGHT = 111 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - A	2x4 DRY	1650F 1.5E	SPF
A - D	2x4 DRY	2100F 1.8E	SPF
D - F	2x4 DRY	1650F 1.5E	SPF
G - F	2x4 DRY	1650F 1.5E	SPF
M - J	2x4 DRY	1650F 1.5E	SPF
J - G	2x4 DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT H - F	2x3 DRY	No.2 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ UPLIFT
M	2353	0	2353	0
G	2353	0	2353	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
G	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
DL = 5.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/728 (0.49")

CSI: TC=0.90/1.00 (B-C:1), BC=0.58/1.00 (H-I:1), WB=0.88/1.00 (A-L:1), SSI=0.44/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747
MT18HS	586	403	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.79 (H) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TMVW-t	MT20	3.5	6.0	1.50	3.00
C	TMW+w	MT20	1.5	4.0		
D	TTWW-m	MT20	4.0	9.0	1.00	3.50
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	9.0	1.75	3.00
G	BMV1+P	MT20	3.0	4.0	2.25	1.50
H	BMVW-t	MT20	5.0	8.0	2.50	2.50
I	BMVW-t	MT20	3.0	4.0		
J	BS-t	MT18HS	3.25	12.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	5.0	8.0	2.25	2.00
M	BMV1+P	MT20	2.0	6.0		

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	C H O R D S		W E B S	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				
M-A	-2303 / 0	0.0	0.0	6.35
A-B	-3399 / 0	-139.4	0.44 (1)	3.80
B-C	-4718 / 0	-139.4	0.90 (1)	3.20
C-D	-4719 / 0	-139.4	0.84 (1)	3.29
D-E	-4282 / 0	-139.4	0.62 (1)	3.35
E-F	-4664 / 0	-139.4	0.67 (1)	3.20
G-F	-2303 / 0	0.0	0.15 (1)	6.35
M-L	0 / 0	-17.5	0.15 (4)	10.00
L-K	0 / 3399	-17.5	0.46 (1)	10.00
K-J	0 / 4047	-17.5	0.53 (1)	10.00
J-I	0 / 4047	-17.5	0.53 (1)	10.00
I-H	0 / 4446	-17.5	0.58 (1)	10.00
H-G	0 / 0	-17.5	0.09 (4)	10.00

CHORDS

WEBS

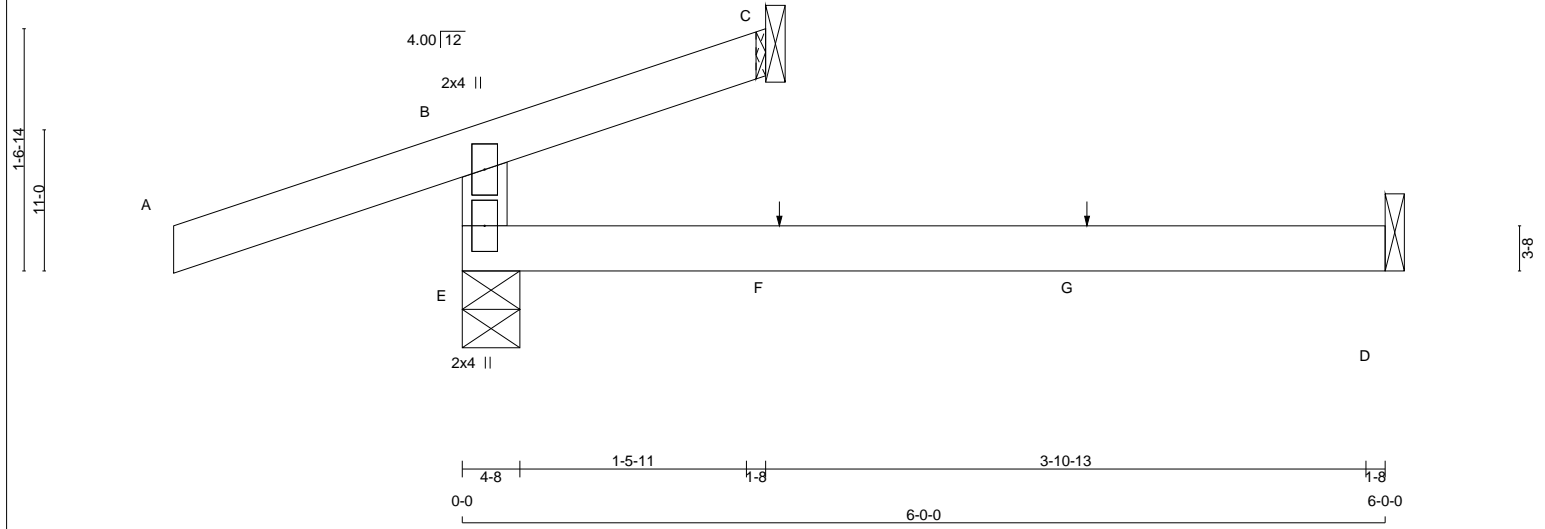
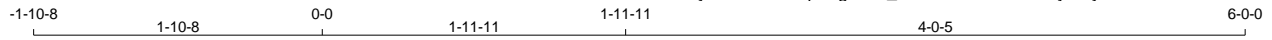


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE

Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.com and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 1650F 1.5E SPF
 E - D 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	558	0	558	0	4-8	1-8
C	52	0	52	0	1-8	1-8
D	38	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	383	317 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	38	17 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	31	0 / -10	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
E-B	-505 / 0	0.0	0.0	0.10 (1)	7.81		
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00		
B-C	-32 / 5	-139.4	-139.4	0.23 (5)	6.25		
E-F	0 / 0	-17.5	-17.5	0.11 (4)	10.00		
F-G	0 / 0	-17.5	-17.5	0.11 (4)	10.00		
G-D	0 / 0	-17.5	-17.5	0.11 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	41	BACK	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

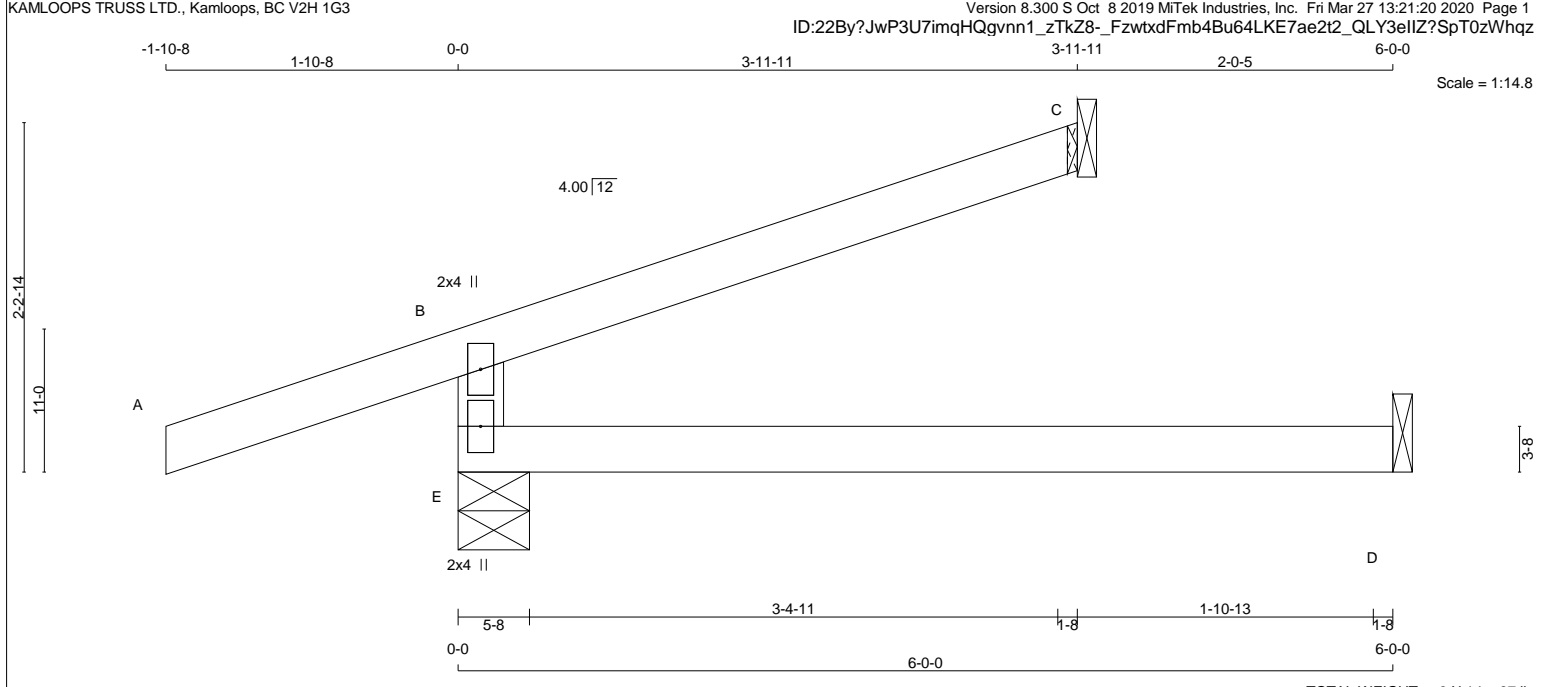
JSI GRIP= 0.27 (B) (INPUT = 0.90)
 JSI METAL= 0.15 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 6 X 14 = 87 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 1650F 1.5E SPF
 E - D 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	BRG	BRG
E	676	0	676	0	0	5-8	1-8	
C	208	0	208	0	0	1-8	1-8	
D	42	0	48	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS						
		1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	467	373 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0	
C	141	126 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0	
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC	MAX CSI (LC)
FR-TO			FROM	TO	FR-TO		LENGTH	FR-TO
E-B	-614 / 0	0.0	0.0	0.12 (4)	7.81			
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00			
B-C	-22 / 0	-139.4	-139.4	0.31 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.12/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

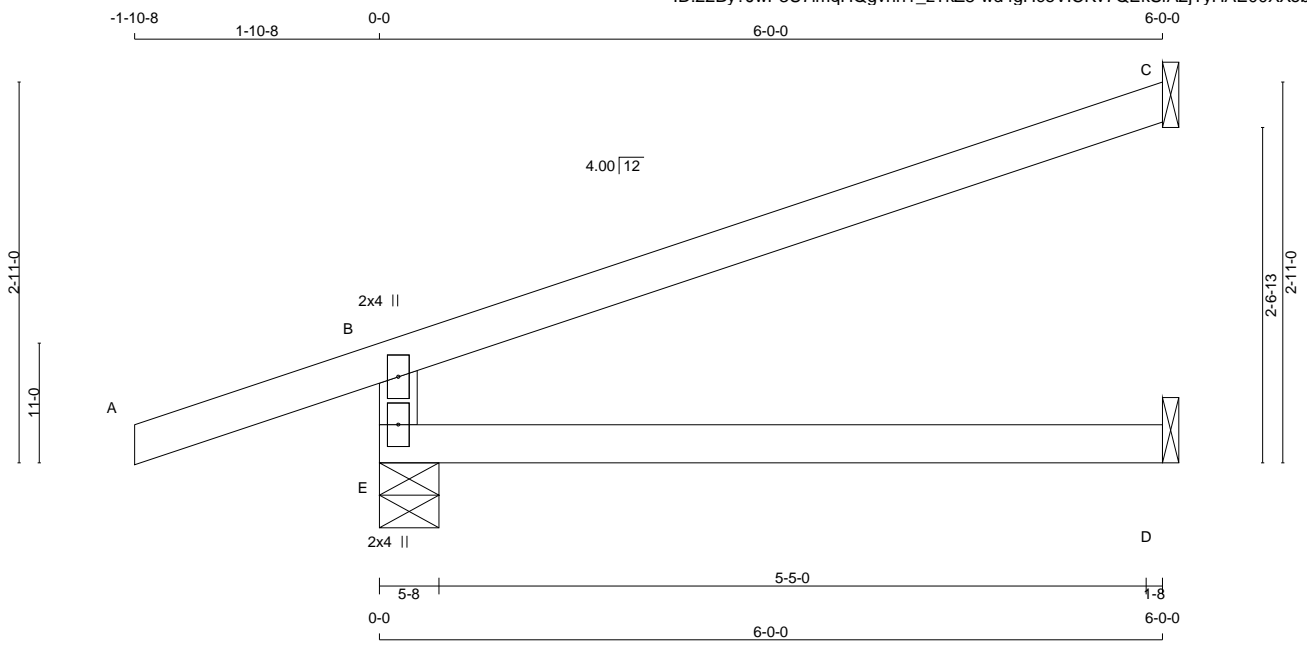
JSI GRIP= 0.32 (B) (INPUT = 0.90)
 JSI METAL = 0.18 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 40 X 17 = 676 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 1650F 1.5E SPF
 E - D 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	853	0	853	0	5-8	1-8
C	314	0	314	0	1-8	1-8
D	42	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	586	480 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
C	213	190 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO
E-B	-790 / 0	0.0	0.0 0.12 (4)	E-B	7.81		
A-B	0 / 42	-139.4	-139.4 0.30 (1)	A-B	6.25		
B-C	-33 / 0	-139.4	-139.4 0.72 (1)	B-C			
E-D	0 / 0	-17.5	-17.5 0.12 (4)	E-D			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.12/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.39/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

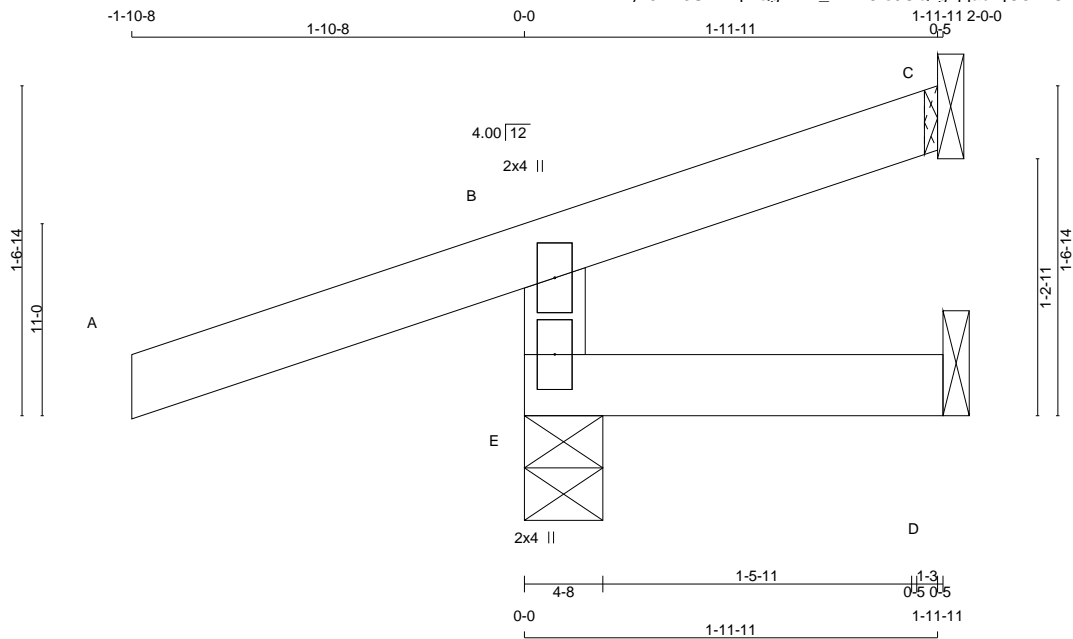
JSI GRIP= 0.42 (B) (INPUT = 0.90)
 JSI METAL= 0.23 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 6 X 7 = 45 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 1650F 1.5E SPF
 E - D 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	553	0	553	0	4-8	1-8
C	43	0	43	0	-75	1-8
D	-17	0	15	0	-33	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	377	325 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	29	25 / -51	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	-10	0 / -28	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB. FACTORED MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-500 / 0	0.0	0.0	0.10 (5)	7.81		
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00		
B-C	-30 / 0	-139.4	-139.4	0.21 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.10 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.
 (55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.10/1.00 (D-E:5) ,
 WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP (PSI)	DRY (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		788	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

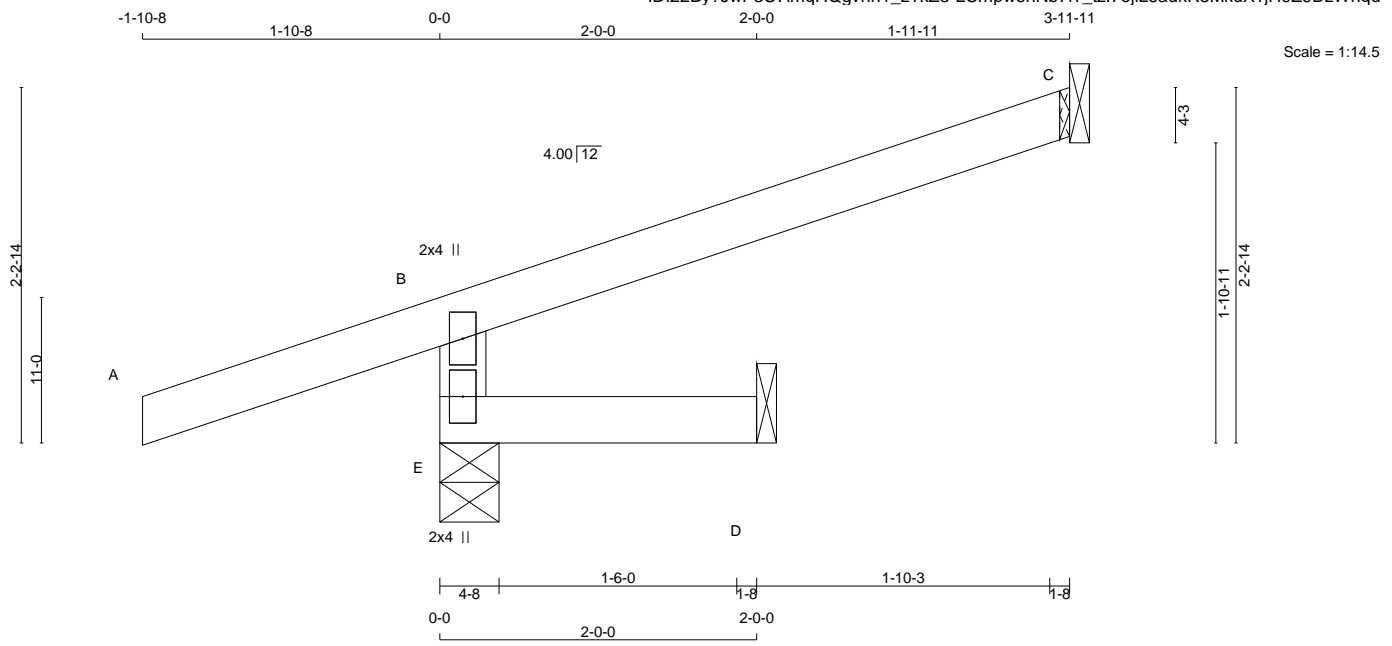
JSI GRIP= 0.26 (B) (INPUT = 0.90)
 JSI METAL= 0.15 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 6 X 10 = 59 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	1650F 1.5E	SPF
A - C	2x4	1650F 1.5E	SPF
E - D	2x4	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG UPLIFT	REQRD BRG IN-SX
E	634	0	634	0	4-8	1-8
C	208	0	208	0	1-8	1-8
D	15	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERMLIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
E	432	373 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	141	126 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC	MAX LENGTH
FR-TO					FR-TO			
E-B	-614 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00			
B-C	-22 / 0	-139.4	-139.4	0.31 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

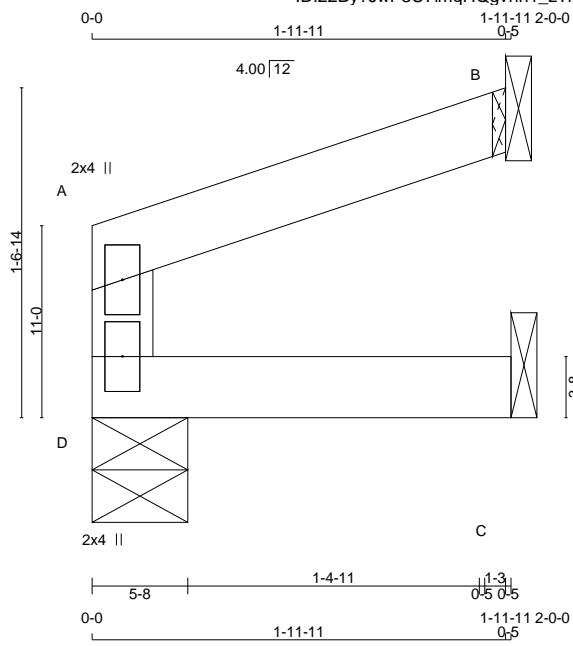
JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 5 lb [M][F]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 D - A 2x4 DRY 1650F 1.5E SPF
 A - B 2x4 DRY 1650F 1.5E SPF
 D - C 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	155	0	155	0	5-8	1-8
B	127	0	127	0	1-8	1-8
C	28	0	28	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
D	107	84 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
B	86	76 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
C	21	7 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO					FR-TO			
D- A	-148 / 0	0.0	0.0	0.02 (1)	7.81			
A- B	-3 / 0	-139.4	-139.4	0.05 (1)	10.00			
D- C	0 / 0	-17.5	-17.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.03/1.00 (C-D:1)
 , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

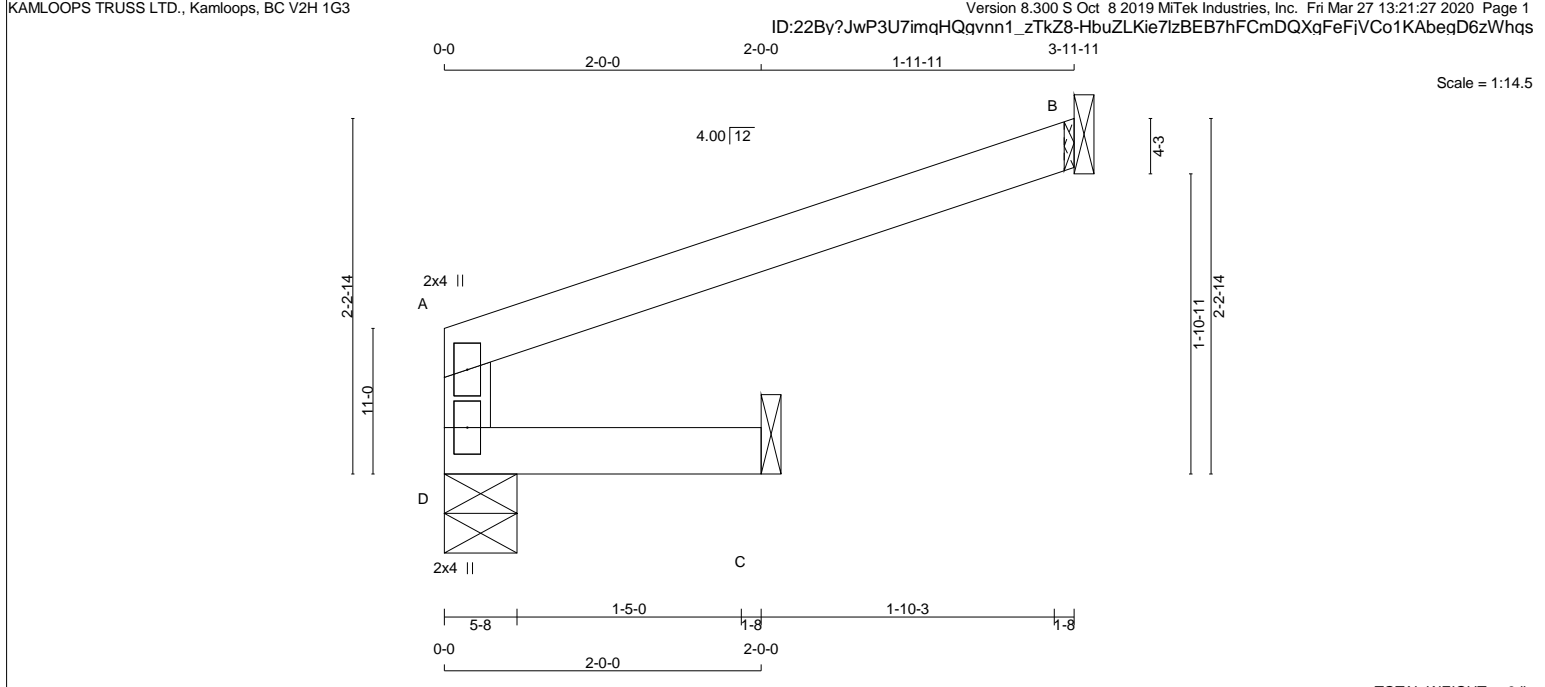
JSI GRIP= 0.08 (A) (INPUT = 0.90)
 JSI METAL= 0.04 (A) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



Scale = 1:14.5

TOTAL WEIGHT = 8 lb [M][F]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 D - A 2x4 DRY 1650F 1.5E SPF
 A - B 2x4 DRY 1650F 1.5E SPF
 D - C 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	259	0	259	0	5-8	1-8
B	241	0	241	0	1-8	1-8
C	88	0	88	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
D	178	146 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	164	146 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
C	62	43 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FR-TO
D-A	-312 / 0	0.0	0.0	0.13 (1)	7.81			
A-B	-11 / 0	-139.4	-139.4	0.22 (1)	6.25			
D-C	0 / 0	-17.5	-17.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (A-B:1), BC=0.16/1.00 (C-D:1)
 , WB=0.00/1.00 (n/a:0), SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE (PSI)	GRIP(DRY) (PLI)	SHEAR (PLI)	SECTION (PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

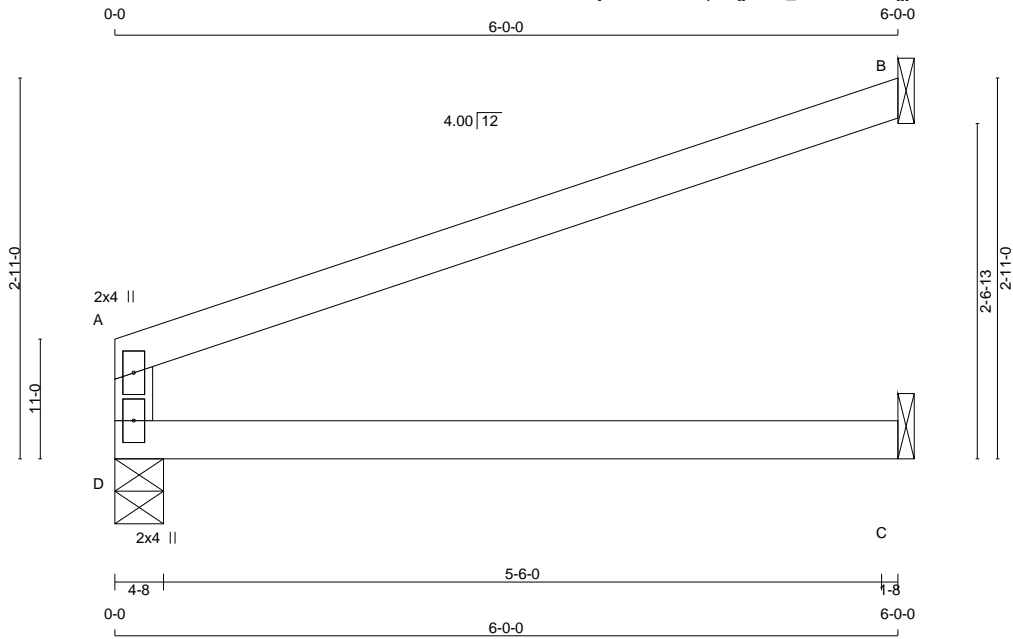
JSI GRIP= 0.17 (A) (INPUT = 0.90)
 JSI METAL= 0.09 (A) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 2 X 15 = 29 lb [M][F]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 D - A 2x4 DRY 1650F 1.5E SPF
 A - B 2x4 DRY 1650F 1.5E SPF
 D - C 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	471	0	471	0	4-8	1-8
B	377	0	377	0	1-8	1-8
C	94	0	94	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD
D	326	254 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
B	257	225 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	69	28 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO
FR-TO								
D- A	-459 / 0	0.0	0.0	0.22 (1)			7.81	
A- B	-13 / 0	-139.4	-139.4	0.58 (1)			6.25	
D- C	0 / 0	-17.5	-17.5	0.29 (1)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/ 851 (0.08")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/ 451 (0.16")

CSI: TC=0.58/1.00 (A-B:1) , BC=0.29/1.00 (C-D:1)
 , WB=0.00/1.00 (n/a:0) , SSI=0.34/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

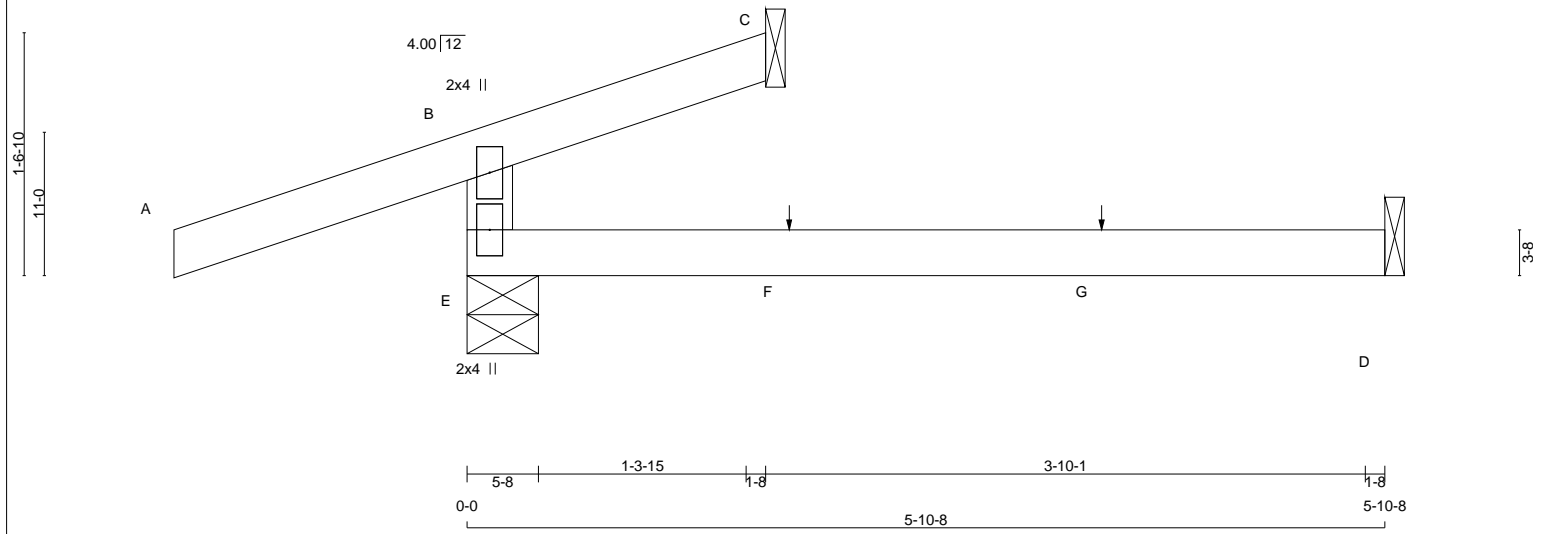
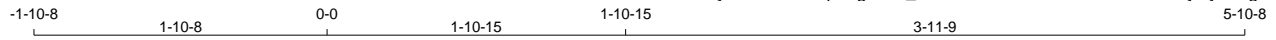
JSI GRIP= 0.24 (A) (INPUT = 0.90)
 JSI METAL= 0.14 (A) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY 1650F 1.5E SPF
 A - C 2x4 DRY 1650F 1.5E SPF
 E - D 2x4 DRY 1650F 1.5E SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	557	0	557	0	5-8	1-8
C	44	0	44	0	1-8	1-8
D	37	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD
E	382	316 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
C	32	12 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
D	30	0 / -11	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
E-B	-505 / 0	0.0	0.0	0.10 (1)	7.81		
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00		
B-C	-33 / 5	-139.4	-139.4	0.23 (5)	6.25		
E-F	0 / 0	-17.5	-17.5	0.10 (4)	10.00		
F-G	0 / 0	-17.5	-17.5	0.10 (4)	10.00		
G-D	0 / 0	-17.5	-17.5	0.10 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL	HEEL	CONN.
F	2-0-12	1	1	41	FRONT	VERT	---	---	C1
G	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.10/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

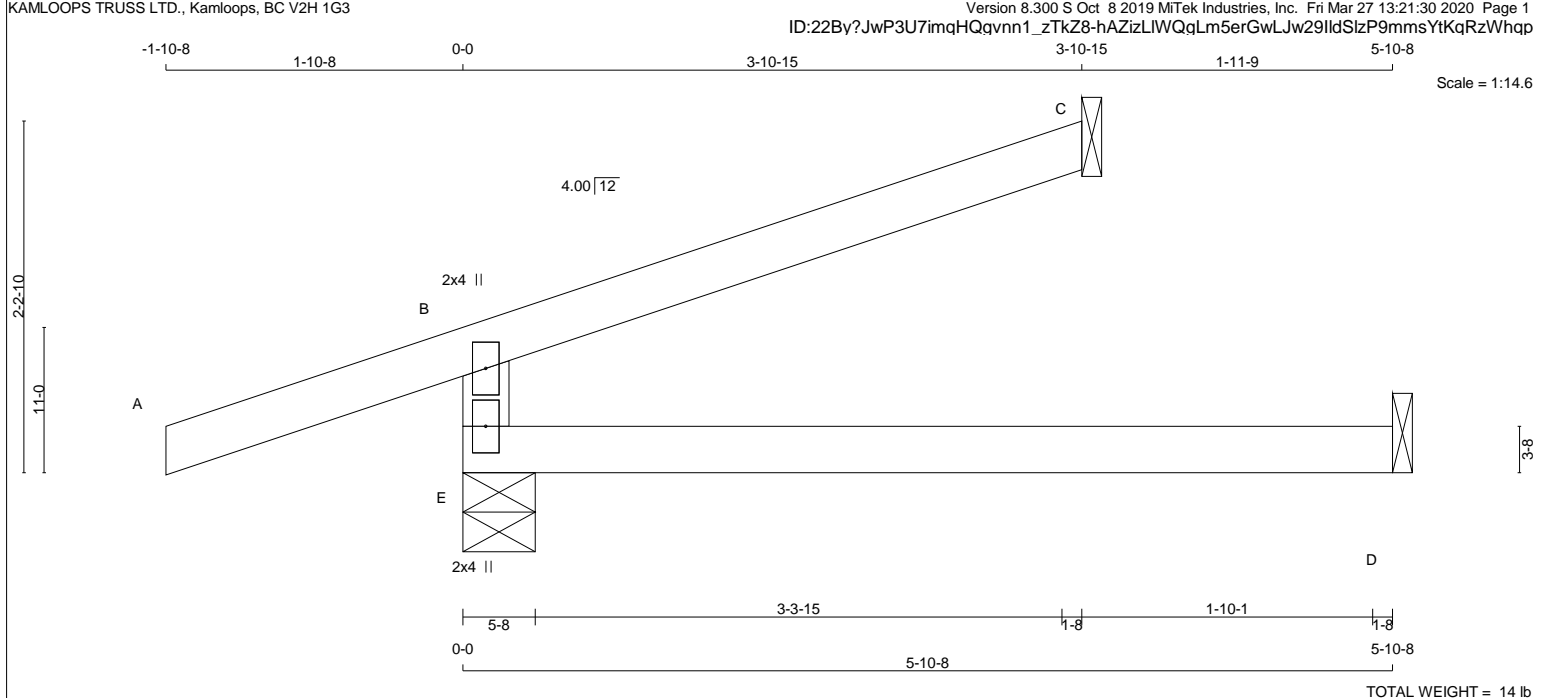
JSI GRIP= 0.27 (B) (INPUT = 0.90)
 JSI METAL= 0.15 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	1650F 1.5E	SPF
A - C	2x4 DRY	1650F 1.5E	SPF
E - D	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	BRG	BRG
E	670	0	670	0	5-8	1-8		
C	205	0	205	0	1-8	1-8		
D	42	0	47	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	462	369 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	139	124 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	33	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO
E-B	-608 / 0	0.0	0.0 0.11 (4)	E-B	7.81	10.00	
A-B	0 / 42	-139.4	-139.4 0.30 (1)	A-B	6.25	10.00	
B-C	-22 / 0	-139.4	-139.4 0.30 (1)	B-C			
E-D	0 / 0	-17.5	-17.5 0.11 (4)	E-D			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4)
 , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

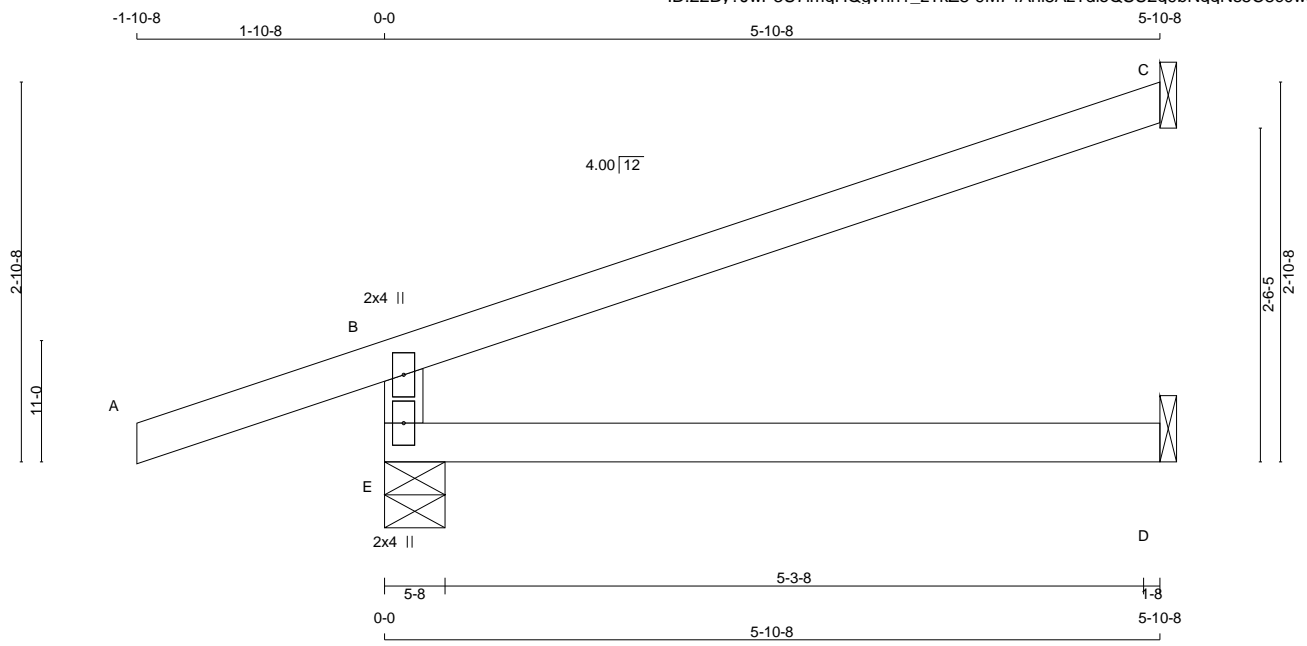
PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
 JSI METAL= 0.18 (B) (INPUT = 1.00)





TOTAL WEIGHT = 5 X 17 = 83 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	1650F 1.5E	SPF
A - C	2x4 DRY	1650F 1.5E	SPF
E - D	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	841	0	841	0	5-8	1-8
C	307	0	307	0	1-8	1-8
D	42	0	47	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	578	473 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	208	186 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
D	33	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-779 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00		
B-C	-32 / 0	-139.4	-139.4	0.69 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.11 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

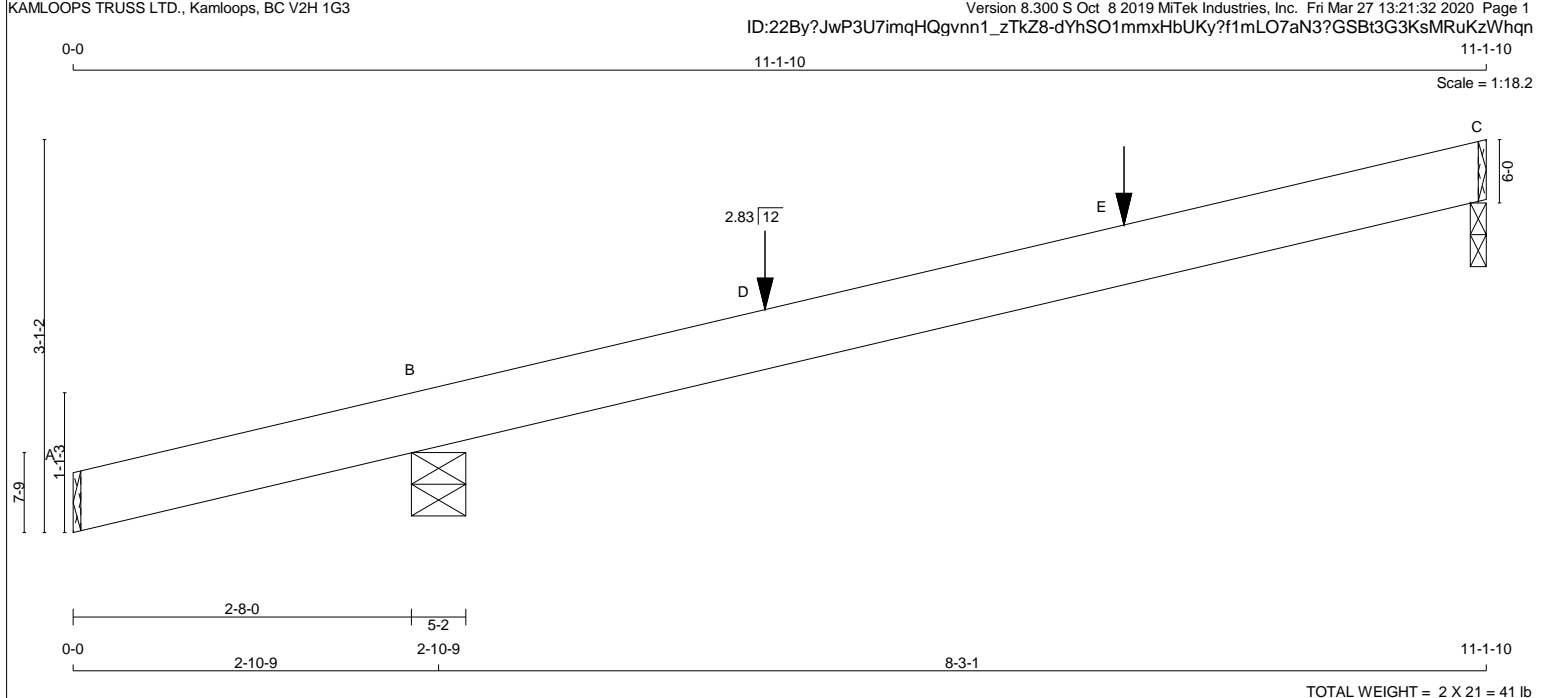
JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 2 X 21 = 41 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x6 DRY
LUMBER 1650F 1.5E
DESCR. SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	319	0	319	0	1-8	1-8
B	256	0	256	0	5-2	5-2

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C, B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
C	217	189/0	0/0	0/0	0/0	29/0	0/0
B	176	145/0	0/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 0.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

MEMB.	CHORDS		W E B S				
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD (LC1)	MAX. (LC)	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. FACTORED (LC)
FR-TO			FROM	TO	LENGTH	FR-TO	
A-B	0/4	-11.6	-11.6	0.03	(1)	10.00	
B-D	0/11	-11.6	-11.6	0.44	(1)	6.25	
D-E	0/11	-11.6	-11.6	0.44	(1)	6.25	
E-C	0/11	-11.6	-11.6	0.44	(1)	6.25	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-1	-35	-35	---	FRONT	VERT	TOTAL	---	C1
D	5-6-1	-26	-26	88	BACK	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	BACK	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 2.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/567 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/374 (0.27")

CSI: TC=0.44/1.00 (B-C:1), BC=0.00/1.00 (n/a:0), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

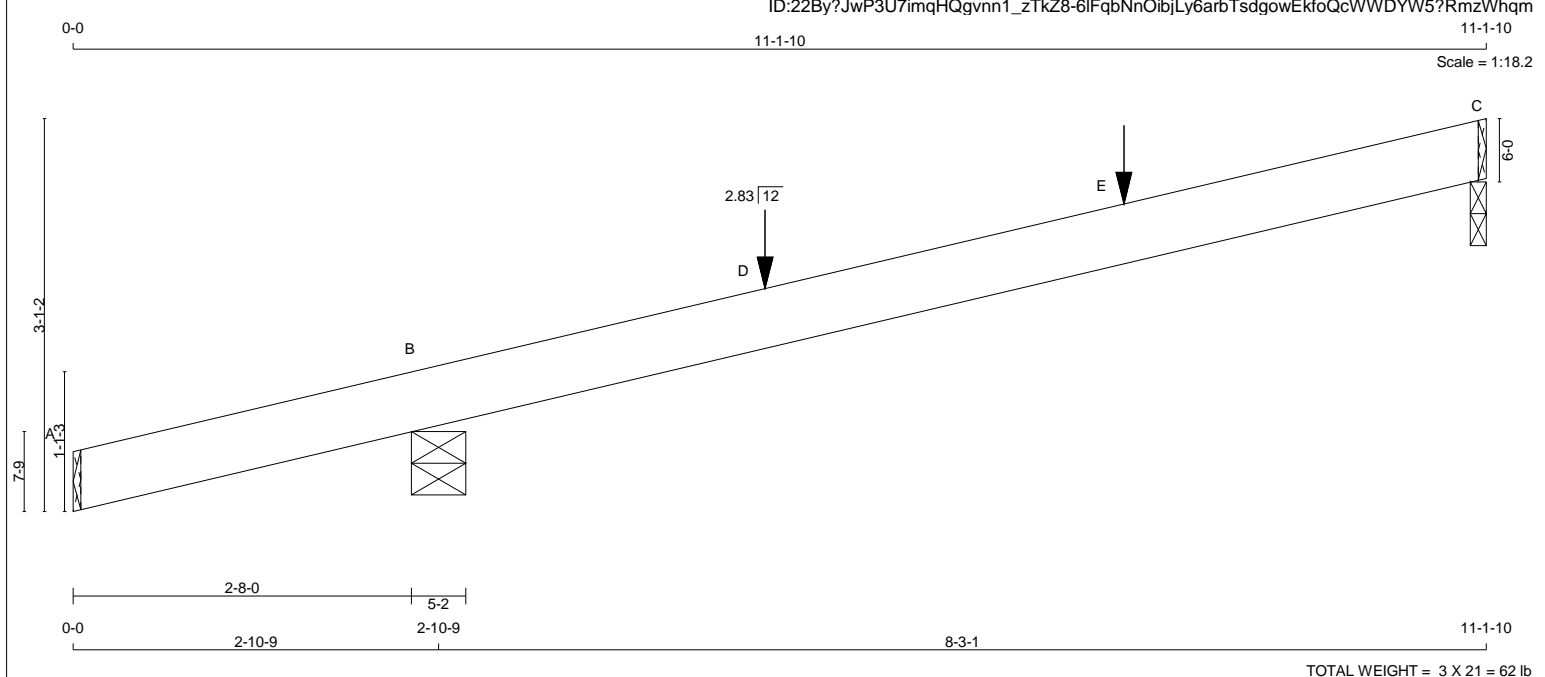
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 3 X 21 = 62 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF
 A - C 2x6 DRY 1650F 1.5E
 DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	319	0	319	0	1-8	1-8
B	256	0	256	0	5-2	5-2

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C, B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	217	189/0	0/0	0/0	0/0	29/0	0/0
B	176	145/0	0/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 0.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

MEMB.	CHORDS		W E B S			
	MAX. FORCE (LBS)	FACTORED (PLF)	MAX. VERT. LOAD (LC1)	MAX. LIVE	MAX. PERM. LIVE	MAX. UNBRACED LENGTH FR-TO
A-B	0/4	-11.6	-11.6	0.03 (1)	10.00	
B-D	0/11	-11.6	-11.6	0.44 (1)	6.25	
D-E	0/11	-11.6	-11.6	0.44 (1)	6.25	
E-C	0/11	-11.6	-11.6	0.44 (1)	6.25	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-1	-35	-35	---	BACK	VERT	TOTAL	---	C1
D	5-6-1	-26	-26	88	FRONT	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	FRONT	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 2.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
 CALCULATED VERT. DEFL.(LL) = L/567 (0.18")
 ALLOWABLE DEFL.(TL)= L/360 (0.28")
 CALCULATED VERT. DEFL.(TL) = L/374 (0.27")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.00/1.00 (n/a:0) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

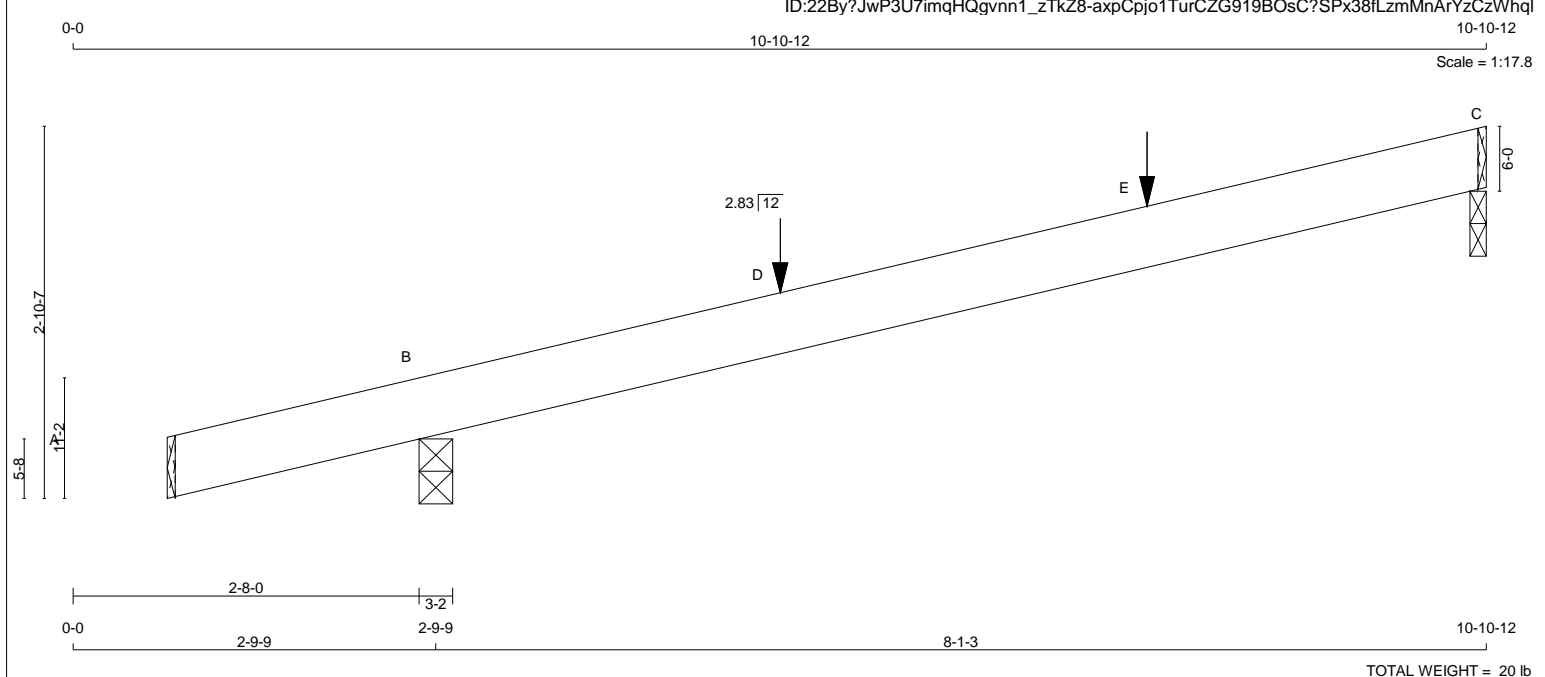
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN


PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

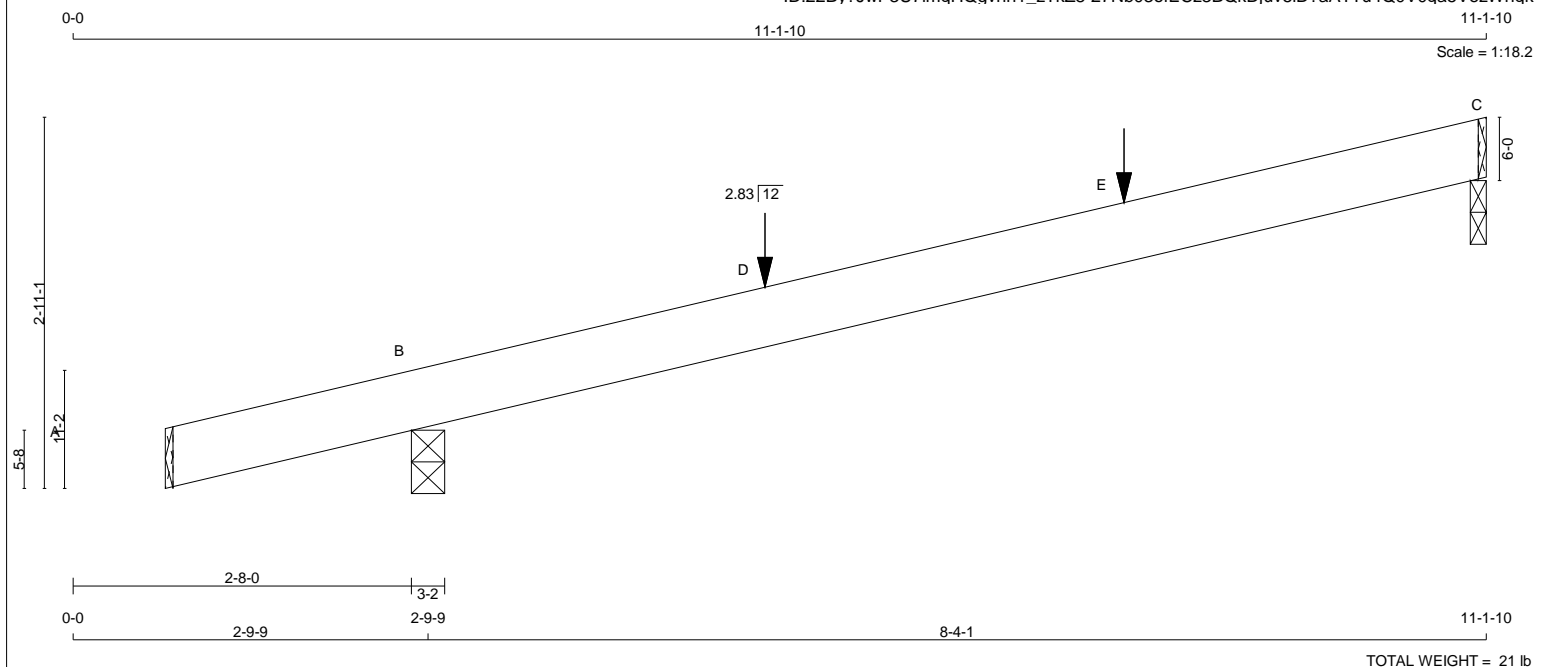


<p>LUMBER N.L.G.A. RULES CHORDS SIZE A - C 2x6 DRY LUMBER 1650F 1.5E DESCR. SPF</p> <p>DRY: SEASONED LUMBER.</p>		<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</p> <p>BEARINGS</p> <table border="1"> <thead> <tr> <th>JT</th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>REQRD BRG</th> </tr> <tr> <th></th> <th>VERT</th> <th>HORZ</th> <th>IN-SX</th> <th>IN-SX</th> </tr> </thead> <tbody> <tr> <td>C</td> <td>323</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> <tr> <td>B</td> <td>237</td> <td>0</td> <td>3-2</td> <td>3-2</td> </tr> </tbody> </table> <p>BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C, B</p> <p>UNFACTORED REACTIONS</p> <table border="1"> <thead> <tr> <th>JT</th> <th>1ST LCASE</th> <th>MAX./MIN. SNOW</th> <th>MIN. LIVE</th> <th>PERM.LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> </thead> <tbody> <tr> <td>C</td> <td>220</td> <td>191 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>29 / 0</td> <td>0 / 0</td> </tr> <tr> <td>B</td> <td>163</td> <td>134 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>29 / 0</td> <td>0 / 0</td> </tr> </tbody> </table> <p>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B</p> <p>BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 0.00 FT OR RIGID CEILING DIRECTLY APPLIED.</p> <p>ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.</p> <p>LOADING TOTAL LOAD CASES: (7)</p> <table border="1"> <thead> <tr> <th colspan="2">CHORDS</th> <th colspan="2">W E B S</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>MAX. FACTORED UNBRAC LENGTH FR-TO</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 4</td> <td>-11.6</td> <td>-11.6 0.02 (1) 10.00</td> </tr> <tr> <td>B-D</td> <td>0 / 14</td> <td>-11.6</td> <td>-11.6 0.41 (1) 6.25</td> </tr> <tr> <td>D-E</td> <td>0 / 14</td> <td>-11.6</td> <td>-11.6 0.41 (1) 6.25</td> </tr> <tr> <td>E-C</td> <td>0 / 14</td> <td>-11.6</td> <td>-11.6 0.41 (1) 6.25</td> </tr> </tbody> </table>		JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG		VERT	HORZ	IN-SX	IN-SX	C	323	0	1-8	1-8	B	237	0	3-2	3-2	JT	1ST LCASE	MAX./MIN. SNOW	MIN. LIVE	PERM.LIVE	WIND	DEAD	SOIL	C	220	191 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	B	163	134 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	CHORDS		W E B S		MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO	FR-TO				A-B	0 / 4	-11.6	-11.6 0.02 (1) 10.00	B-D	0 / 14	-11.6	-11.6 0.41 (1) 6.25	D-E	0 / 14	-11.6	-11.6 0.41 (1) 6.25	E-C	0 / 14	-11.6	-11.6 0.41 (1) 6.25	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS: TOP CH. LL = 42.3 PSF DL = 5.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 54.3 PSF</p> <p>SPACING = 2.0 IN. C/C</p> <p>THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015</p> <p>THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014</p> <p>(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD</p> <p>ALLOWABLE DEFL.(LL)= L/360 (0.27") CALCULATED VERT. DEFL.(LL) = L/ 624 (0.16") ALLOWABLE DEFL.(TL)= L/360 (0.27") CALCULATED VERT. DEFL.(TL) = L/ 410 (0.24")</p> <p>CSI: TC=0.41/1.00 (B-C:1) , BC=0.00/1.00 (n/a:0) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00</p> <p>COMPANION LIVE LOAD FACTOR = 1.00</p> <p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</p> <p>NAIL VALUES</p> <table border="1"> <thead> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR (PSI)</th> <th>SECTION (PLI)</th> </tr> </thead> <tbody> <tr> <td></td> <td>MAX</td> <td>MIN</td> <td>MAX MIN</td> </tr> </tbody> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.</p>		PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)		MAX	MIN	MAX MIN
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG																																																																																	
	VERT	HORZ	IN-SX	IN-SX																																																																																	
C	323	0	1-8	1-8																																																																																	
B	237	0	3-2	3-2																																																																																	
JT	1ST LCASE	MAX./MIN. SNOW	MIN. LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																														
C	220	191 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0																																																																														
B	163	134 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0																																																																														
CHORDS		W E B S																																																																																			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO																																																																																		
FR-TO																																																																																					
A-B	0 / 4	-11.6	-11.6 0.02 (1) 10.00																																																																																		
B-D	0 / 14	-11.6	-11.6 0.41 (1) 6.25																																																																																		
D-E	0 / 14	-11.6	-11.6 0.41 (1) 6.25																																																																																		
E-C	0 / 14	-11.6	-11.6 0.41 (1) 6.25																																																																																		
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)																																																																																		
	MAX	MIN	MAX MIN																																																																																		



1) **C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.**

This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.



TOTAL WEIGHT = 21 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - C 2x6 DRY 1650F 1.5E
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	321	0	321	0	1-8	1-8
B	253	0	253	0	3-2	3-2

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C, B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
C	219	190/0	0/0	0/0	0/0	29/0	0/0
B	174	143/0	0/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 0.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

MEMB.	CHORDS		W E B S				
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. FACTORED (LC)
FR-TO					LENGTH FR-TO		
A-B	0/4		-11.6	-11.6	0.02 (1)	10.00	
B-D	0/12		-11.6	-11.6	0.44 (1)	6.25	
D-E	0/12		-11.6	-11.6	0.44 (1)	6.25	
E-C	0/12		-11.6	-11.6	0.44 (1)	6.25	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-1	-35	-35	---	FRONT	VERT	TOTAL	---	C1
D	5-6-1	-26	-26	88	BACK	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	BACK	VERT	TOTAL	---	C1
E	8-4-0	-191	-191	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 2.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 557 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 368 (0.27")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.00/1.00 (n/a:0) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

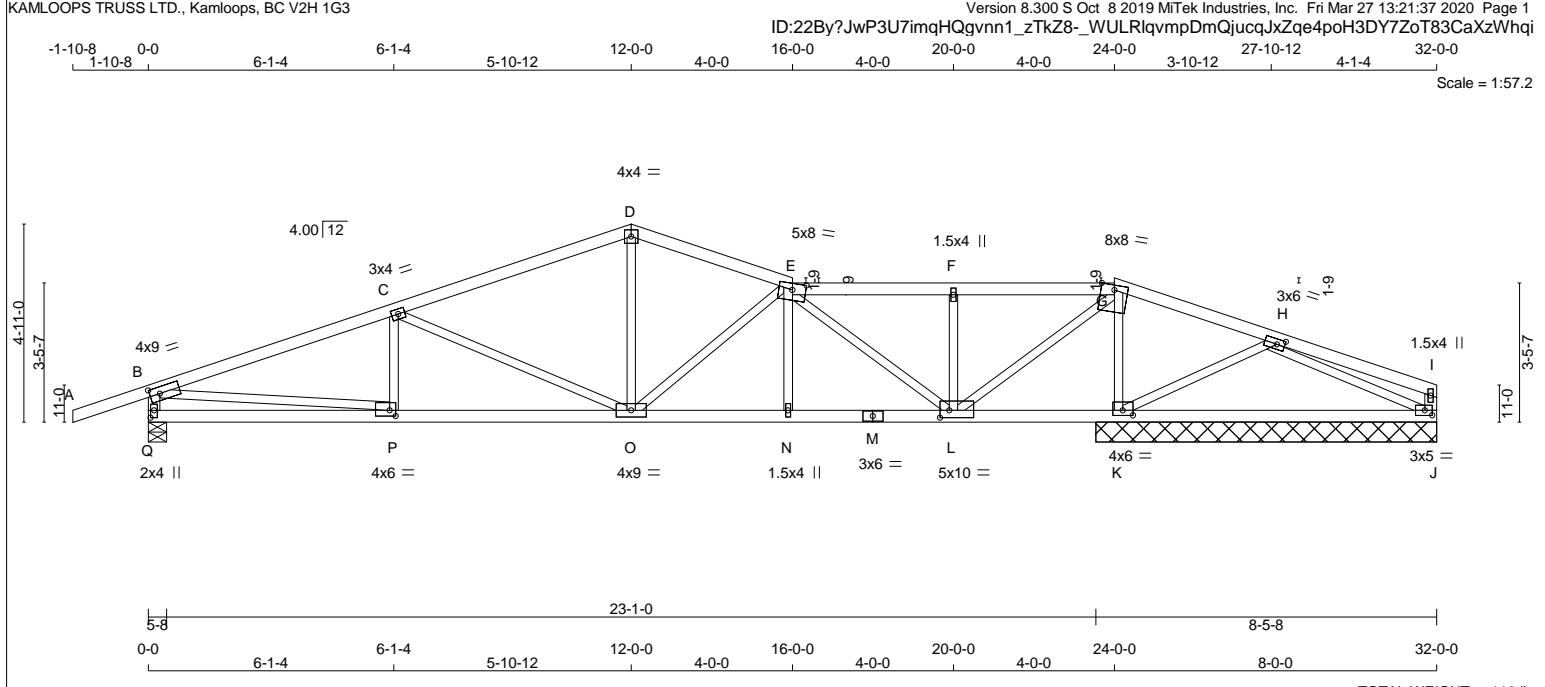
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 119 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
Q - B	2x4	DRY	1650F 1.5E	SPF
J - I	2x4	DRY	1650F 1.5E	SPF
Q - M	2x4	DRY	1650F 1.5E	SPF
M - J	2x4	DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TMVW-t	MT20	3.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TTWWW-m	MT20	5.0	8.0	2.00	4.00
F	TMVW-w	MT20	1.5	4.0		
G	TTWW-m	MT20	8.0	8.0	Edge	
H	TMVW-t	MT20	3.0	6.0	1.50	2.25
I	TMV-p	MT20	1.5	4.0		
J	BMVW1-t	MT20	3.0	5.0	1.50	2.25
K	BMVW1-t	MT20	4.0	6.0	1.50	3.00
L	BMVWV-t	MT20	5.0	10.0	2.25	2.75
M	BS-t	MT20	3.0	6.0		
N	BMVW-w	MT20	1.5	4.0		
O	BMVWV-t	MT20	4.0	9.0		
P	BMVW-t	MT20	4.0	6.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UPLIFT	IN-SX
Q	1908	0	1908	0
K	3523	0	0	8-5-8
J	-144	0	0	-144

PROVIDE ANCHORAGE AT BEARING JOINT J FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1317	1047 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
K	2439	1901 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0
J	-100	0 / -78	0 / 0	0 / 0	0 / 0	0 / -21	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.41 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)		
FR-TO		FROM TO	LENGTH	FR-TO				
A-B	0 / 42	-139.4 -139.4	0.30 (1)	10.00	P-C	-191 / 71	0.03 (1)	
B-C	-3025 / 0	-139.4 -139.4	0.88 (1)	3.43	C-O	-1173 / 0	0.89 (1)	
C-D	-1958 / 0	-139.4 -139.4	0.77 (1)	4.21	O-D	0 / 656	0.15 (1)	
D-E	-1937 / 0	-139.4 -139.4	0.35 (1)	4.99	O-E	-112 / 0	0.05 (1)	
E-F	-532 / 0	-139.4 -139.4	0.30 (1)	6.25	N-E	0 / 79	0.03 (4)	
F-G	-532 / 0	-139.4 -139.4	0.30 (1)	6.25	E-L	-1776 / 0	0.78 (1)	
G-H	0 / 1919	-139.4 -139.4	0.56 (1)	10.00	L-F	-704 / 0	0.14 (1)	
H-I	0 / 23	-139.4 -139.4	0.34 (1)	10.00	L-G	0 / 2940	0.66 (1)	
Q-B	-1857 / 0	0.0	0.0	0.12 (1)	6.89	K-G	-2840 / 0	0.60 (1)
J-I	-208 / 0	0.0	0.0	0.01 (1)	7.81	K-H	-1147 / 0	0.38 (1)
						B-P	0 / 2913	0.66 (1)
						H-J	0 / 922	0.21 (1)
Q-P	0 / 0	-17.5	-17.5	0.12 (4)	10.00			
P-O	0 / 2898	-17.5	-17.5	0.39 (1)	10.00			
O-N	0 / 1916	-17.5	-17.5	0.28 (1)	10.00			
N-M	0 / 1914	-17.5	-17.5	0.28 (1)	10.00			
M-L	0 / 1914	-17.5	-17.5	0.28 (1)	10.00			
L-K	-1759 / 0	-17.5	-17.5	0.22 (4)	5.41			
K-J	-829 / 0	-17.5	-17.5	0.22 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL)= L/360 (0.80")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.88/1.00 (B-C:1) , BC=0.39/1.00 (O-P:1) , WB=0.89/1.00 (C-O:1) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
 JSI METAL= 0.69 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME ST1	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 63 of 75
--------------------	-------------------	---------------	----------	--------------------------------	----------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:37 2020 Page 2

ID:22By?JwP3U7imqHQgvnn1_zTkZ8- WULRIqympDmQjucqJxZqe4poH3DY7ZoT83CaXzWhqi

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
Q	BMV1+p	MT20	2.0	4.0	2.25 1.00

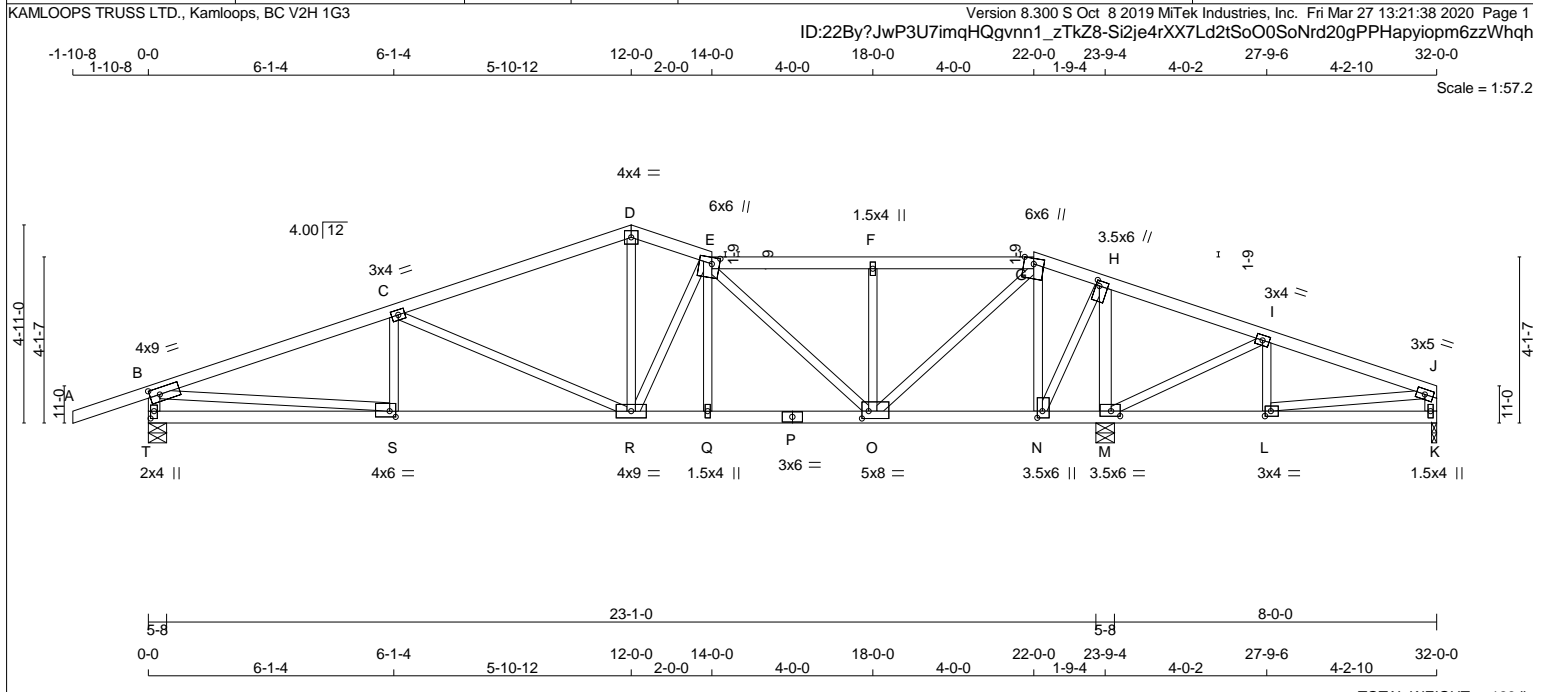
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - E	2x4	DRY 1650F 1.5E	SPF
E - G	2x4	DRY 1650F 1.5E	SPF
G - J	2x4	DRY 1650F 1.5E	SPF
T - B	2x4	DRY 1650F 1.5E	SPF
K - J	2x4	DRY 1650F 1.5E	SPF
T - P	2x4	DRY 1650F 1.5E	SPF
P - K	2x4	DRY 1650F 1.5E	SPF
ALL WEBS EXCEPT M - H	2x3	DRY No.2	SPF
M - H	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TMVW-t	MT20	3.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TTWWW+m	MT20	6.0	6.0	2.00	2.25
F	TMVW+w	MT20	1.5	4.0		
G	TTWW+m	MT20	6.0	6.0	Edge	
H	TMVW+t	MT20	3.5	6.0	1.50	1.00
I	TMVW-t	MT20	3.0	4.0		
J	TMVW-t	MT20	3.0	5.0		
K	BMV1+p	MT20	1.5	4.0		
L	BMVW-t	MT20	3.0	4.0	1.50	1.75
M	BMVW1-t	MT20	3.5	6.0	1.50	2.75
N	BMVW+t	MT20	3.5	6.0	2.00	1.50
O	BMVW-t	MT20	5.0	8.0	2.25	2.00



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
T	1905 0	1905 0	5-8	2-13
M	3393 0	3393 0	5-8	4-5
K	-10 0	0 0	-10 1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT K FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1315	1046 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
M	2349	1828 / 0	0 / 0	0 / 0	0 / 0	521 / 0	0 / 0
K	-7	0 / -5	0 / 0	0 / 0	0 / 0	0 / -3	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.84 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO					FR-TO			
A-B	0 / 42	-139.4	-139.4	0.30 (1)	10.00	S-C	-191 / 71	0.03 (1)
B-C	-3017 / 0	-139.4	-139.4	0.60 (1)	3.90	C-R	-1173 / 0	0.89 (1)
C-D	-1950 / 0	-139.4	-139.4	0.54 (1)	4.72	R-D	0 / 818	0.18 (1)
D-E	-1938 / 0	-139.4	-139.4	0.08 (1)	5.36	R-E	-256 / 0	0.08 (1)
E-F	-1134 / 0	-139.4	-139.4	0.23 (1)	6.25	Q-E	0 / 36	0.01 (4)
F-G	-1134 / 0	-139.4	-139.4	0.23 (1)	6.25	E-O	-1123 / 0	0.59 (1)
G-H	0 / 546	-139.4	-139.4	0.27 (1)	10.00	O-F	-721 / 0	0.19 (1)
H-I	0 / 1599	-139.4	-139.4	0.48 (1)	10.00	O-G	0 / 2276	0.51 (1)
I-J	0 / 628	-139.4	-139.4	0.37 (1)	10.00	N-G	-1968 / 0	0.53 (1)
T-B	-1854 / 0	0.0	0.0	0.12 (1)	6.90	N-H	0 / 2184	0.49 (1)
K-J	0 / 44	0.0	0.0	0.01 (4)	10.00	M-H	-2789 / 0	0.30 (1)
T-S	0 / 0	-17.5	-17.5	0.12 (4)	10.00	L-I	-1059 / 0	0.35 (1)
S-R	0 / 2890	-17.5	-17.5	0.40 (1)	10.00	B-S	0 / 2905	0.65 (1)
R-Q	0 / 1938	-17.5	-17.5	0.29 (1)	10.00	L-J	-583 / 0	0.17 (1)
Q-P	0 / 1938	-17.5	-17.5	0.27 (1)	10.00			
P-O	0 / 1938	-17.5	-17.5	0.27 (1)	10.00			
O-N	-499 / 0	-17.5	-17.5	0.04 (4)	6.25			
N-M	-1522 / 0	-17.5	-17.5	0.10 (1)	5.84			
M-L	-577 / 0	-17.5	-17.5	0.09 (1)	6.25			
L-K	0 / 0	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.40/1.00 (R-S:1) , WB=0.89/1.00 (C-R:1) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
 JSI METAL= 0.69 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.

JOB NAME 200656	TRUSS NAME ST2	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 65 of 75
--------------------	-------------------	---------------	----------	--------------------------------	----------------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3

Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:38 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-Si2je4rXX7Ld2tSoO0SoNrd20gPPHapyiopm6zzWhqh

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-t	MT20	3.0	6.0		
Q	BMW+w	MT20	1.5	4.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BMWWW-t	MT20	4.0	6.0	1.75	1.75
T	BMV1+p	MT20	2.0	4.0	2.25	1.00

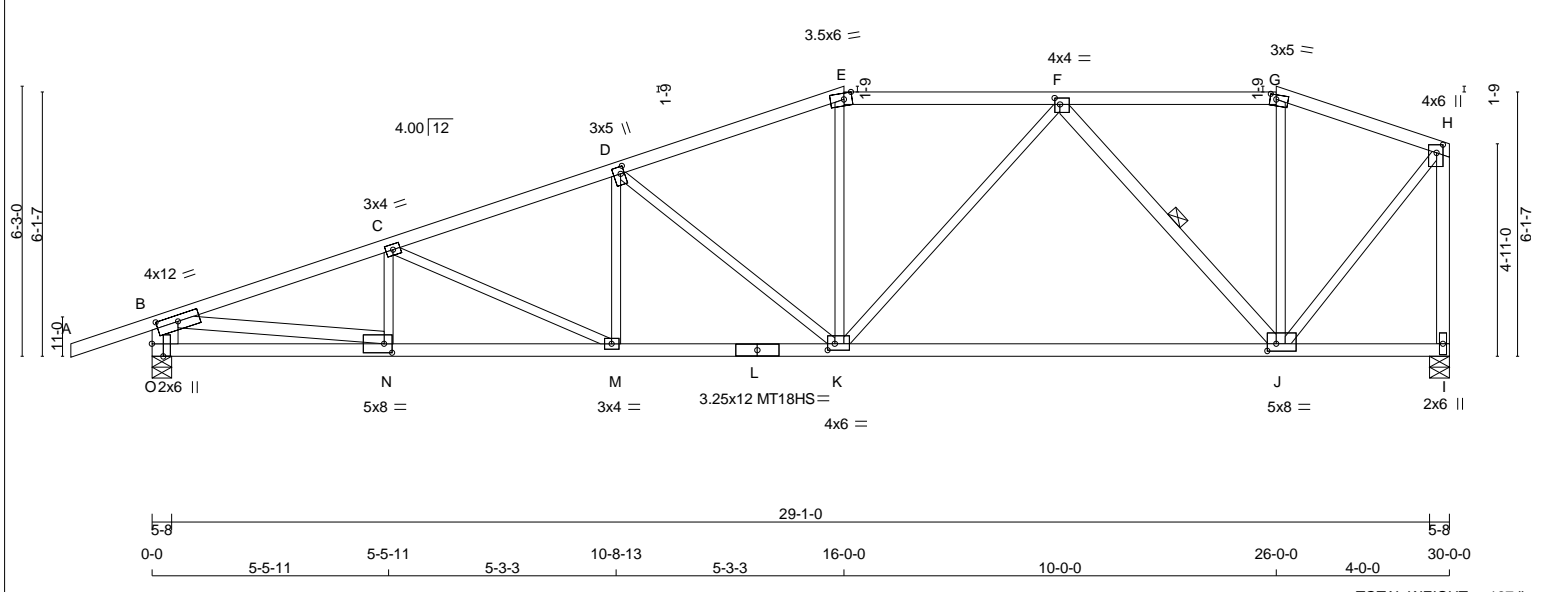
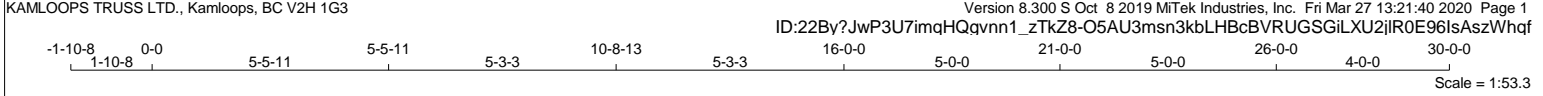
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY 1650F 1.5E	SPF
E - G	2x4	DRY 1650F 1.5E	SPF
G - H	2x4	DRY 1650F 1.5E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
O - B	2x8	DRY 1950F 1.7E	DF
O - L	2x4	DRY 1650F 1.5E	SPF
L - I	2x4	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

F - J	2x4	DRY 1650F 1.5E	SPF
B - N	2x4	DRY 1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	12.0	1.75	6.00
C	TMVW-t	MT20	3.0	4.0		
D	TMVW+t	MT20	3.0	5.0	2.00	1.00
E	TTW-m	MT20	3.5	6.0	Edge	2.25
F	TMVW-t	MT20	4.0	4.0	1.75	1.50
G	TTW-m	MT20	3.0	5.0	1.25	1.75
H	TMVW+p	MT20	4.0	6.0	2.25	1.75
I	BMV1+p	MT20	2.0	6.0		
J	BMVWVW-t	MT20	5.0	8.0	2.00	2.50
K	BMVWVW-t	MT20	4.0	6.0	1.75	2.00
L	BS-t	MT18HS	3.25	12.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	5.0	8.0	2.50	2.25
O	BMV1+p	MT20	2.0	6.0	Edge	3.25



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	INPUT UPLIFT	REQRD BRG IN-SX
O	2621	0	2621	0	0	5-8 2-13
I	2353	0	2353	0	0	5-8 4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS PERMLIVE	WIND	DEAD	SOIL
O	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-J. DBS = 16-0-0. CBF = 185 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-139.4 -139.4	0.30 (1)	10.00	N-C	-410 / 15	0.07 (1)
B-C	-4675 / 0	-139.4 -139.4	0.57 (1)	3.30	C-M	-550 / 0	0.32 (1)
C-D	-4168 / 0	-139.4 -139.4	0.45 (1)	3.62	M-D	0 / 300	0.07 (1)
D-E	-3193 / 0	-139.4 -139.4	0.43 (1)	4.05	D-K	-1206 / 0	0.98 (1)
E-F	-3013 / 0	-139.4 -139.4	0.42 (1)	4.14	K-E	0 / 447	0.10 (1)
F-G	-1380 / 0	-139.4 -139.4	0.38 (1)	5.66	K-F	0 / 627	0.14 (1)
G-H	-1445 / 0	-139.4 -139.4	0.25 (1)	5.76	F-J	-1850 / 0	0.39 (1)
I-H	-2349 / 0	0.0 0.0	0.72 (1)	6.30	J-G	-88 / 49	0.05 (1)
O-B	-2573 / 0	0.0 0.0	0.07 (1)	7.81	J-H	0 / 2136	0.48 (1)
					B-N	0 / 4482	0.52 (1)
O-N	0 / 0	-17.5 -17.5	0.09 (4)	10.00			
N-M	0 / 4454	-17.5 -17.5	0.59 (1)	10.00			
M-L	0 / 3955	-17.5 -17.5	0.60 (1)	10.00			
L-K	0 / 3955	-17.5 -17.5	0.60 (1)	10.00			
K-J	0 / 2598	-17.5 -17.5	0.44 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 42.3 PSF
DL = 5.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/923 (0.39")

CSI: TC=0.72/1.00 (H-I:1), BC=0.60/1.00 (K-M:1), WB=0.98/1.00 (D-K:1), SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX MIN	650 371	1747 788	1987 1873
MT20	650 371	1747 788	1987 1873
MT18HS	586 403	2455 1382	3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200656	ST3	1	1	Wood Creek Const.	67 of 75
KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3				TRUSS DESC.	ST3

Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:40 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-O5AU3msn3kbLHBcBVRUGSGiLXU2jIR0E96lsAszWhqf

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.79 (N) (INPUT = 1.00)

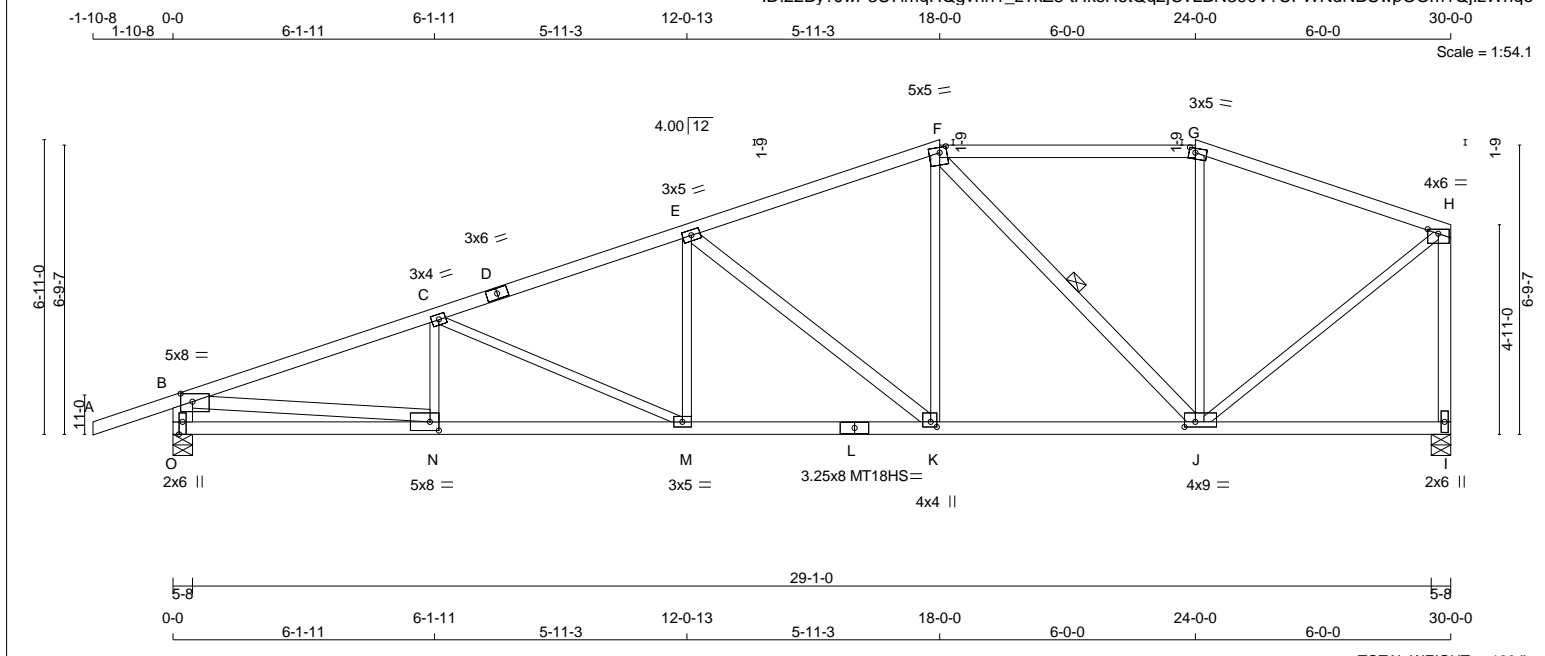


This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use onlywith MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781 N. Lee Street, Alexandria, VA 22314.

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MITek Industries, Inc. Fri Mar 27 13:21:41 2020 Page 1 ID:22By?JwP3U7ImqHqgvnn1_zTkZ8-tHksH6tQq2jCvLBN390V?UFWNuNBuWpOOm1QjizWhqe



TOTAL WEIGHT = 129 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - F	2x4	DRY 1650F 1.5E	SPF
F - G	2x4	DRY 2100F 1.8E	SPF
G - H	2x4	DRY 2100F 1.8E	SPF
I - H	2x4	DRY 1650F 1.5E	SPF
O - B	2x6	DRY 1650F 1.5E	SPF
O - L	2x4	DRY 1650F 1.5E	SPF
L - I	2x4	DRY 1650F 1.5E	SPF

ALL WEBS	EXCEPT	SIZE	DRY	No.2	SPF
E - K	2x4	DRY	1650F 1.5E	SPF	
F - J	2x4	DRY	1650F 1.5E	SPF	
B - N	2x4	DRY	1650F 1.5E	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.25	3.25
C	TMVW-t	MT20	3.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	3.0	5.0		
F	TTWW-m	MT20	5.0	5.0	1.50	2.00
G	TTW-m	MT20	3.0	5.0	1.25	1.75
H	TMVW-p	MT20	4.0	6.0	1.25	3.00
I	BMV1+p	MT20	2.0	6.0		
J	BMVWW-t	MT20	4.0	9.0	1.50	3.00
K	BMVWW-t	MT20	4.0	4.0	1.50	1.75
L	BS-t	MT18HS	3.25	8.0		
M	BMVW-t	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
O	2621 0	2621 0 0	5-8	3-0
I	2353 0	2353 0 0	5-8	4-2

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
O	1811	1431 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1629	1269 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-J. DBS = 20-0-0 . CBF = 160 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-139.4 -139.4	0.30 (1)	10.00	N-C	-350 / 34	0.06 (1)
B-C	-4708 / 0	-139.4 -139.4	0.72 (1)	3.13	C-M	-788 / 0	0.61 (1)
C-D	-3973 / 0	-139.4 -139.4	0.56 (1)	3.57	M-E	0 / 431	0.10 (1)
D-E	-3973 / 0	-139.4 -139.4	0.56 (1)	3.57	E-K	-1487 / 0	0.88 (1)
E-F	-2764 / 0	-139.4 -139.4	0.53 (1)	4.15	K-F	0 / 1029	0.23 (1)
F-G	-1717 / 0	-139.4 -139.4	0.44 (1)	5.47	F-J	-1282 / 0	0.37 (1)
G-H	-1799 / 0	-139.4 -139.4	0.44 (1)	5.38	J-G	-283 / 38	0.23 (1)
I-H	-2312 / 0	0.0 0.0	0.71 (1)	6.33	J-H	0 / 2175	0.49 (1)
O-B	-2568 / 0	0.0 0.0	0.10 (1)	7.23	B-N	0 / 4511	0.53 (1)
O-N	0 / 0	-17.5 -17.5	0.11 (4)	10.00			
N-M	0 / 4489	-17.5 -17.5	0.58 (1)	10.00			
M-L	0 / 3770	-17.5 -17.5	0.49 (1)	10.00			
L-K	0 / 3770	-17.5 -17.5	0.49 (1)	10.00			
K-J	0 / 2590	-17.5 -17.5	0.37 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 42.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
 ALLOWABLE DEFL.(TL)= L/360 (1.00")
 CALCULATED VERT. DEFL.(TL) = L/ 961 (0.37")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.58/1.00 (M-N:1) , WB=0.88/1.00 (E-K:1) , SSI=0.37/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
MT18HS	586	403	2455
	MIN	MIN	MIN
	1987	1873	3163
	3004		

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
 Design valid for use only with MITek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA. 22314.

JOB NAME 200656	TRUSS NAME ST4	QUANTITY 1	PLY 1	JOB DESC. Wood Creek Const.	DRWG NO. 69 of 75	ST4
--------------------	-------------------	---------------	----------	--------------------------------	----------------------	------------

KAMLOOPS TRUSS LTD., Kamloops, BC V2H 1G3 Version 8.300 S Oct 8 2019 MiTek Industries, Inc. Fri Mar 27 13:21:41 2020 Page 2
ID:22By?JwP3U7imqHQgvnn1_zTkZ8-tHksH6tQq2jCvLBN390V?UFWNuNBUpOOm1QjzWhqe

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMW-t	MT20	5.0	8.0	2.50	2.50
O	BMV1-p	MT20	2.0	6.0	Edge	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

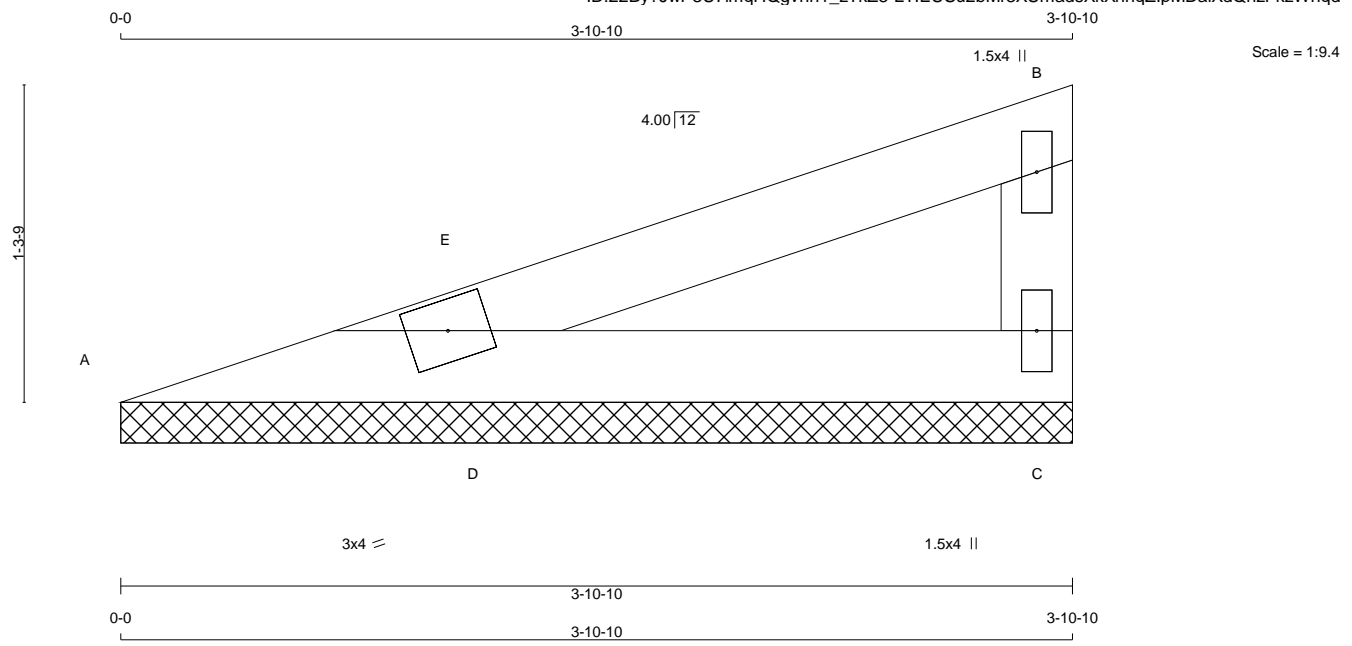
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.80 (N) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss-Plate-Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	1650F 1.5E	SPF
C - B	2x4 DRY	1650F 1.5E	SPF
A - C	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMV+p	MT20	1.5	4.0		
C	BMV1+p	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	305	0	305	0	3-10-10	3-13
C	305	0	305	0	3-10-10	3-13

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
A	211	164 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	211	164 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-E	-16 / 0	-139.4	-139.4 0.13 (1)	6.25	D-E -157 / 6	0.00 (1)
E-B	0 / 15	-139.4	-139.4 0.18 (1)	10.00		
C-B	-207 / 0	0.0	0.0 0.01 (1)	7.81		
A-D	-11 / 0	-17.5	-17.5 0.20 (1)	6.25		
D-C	0 / 0	-17.5	-17.5 0.20 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (B-E:1), BC=0.20/1.00 (A-D:1), WB=0.00/1.00 (D-E:1), SSI=0.15/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

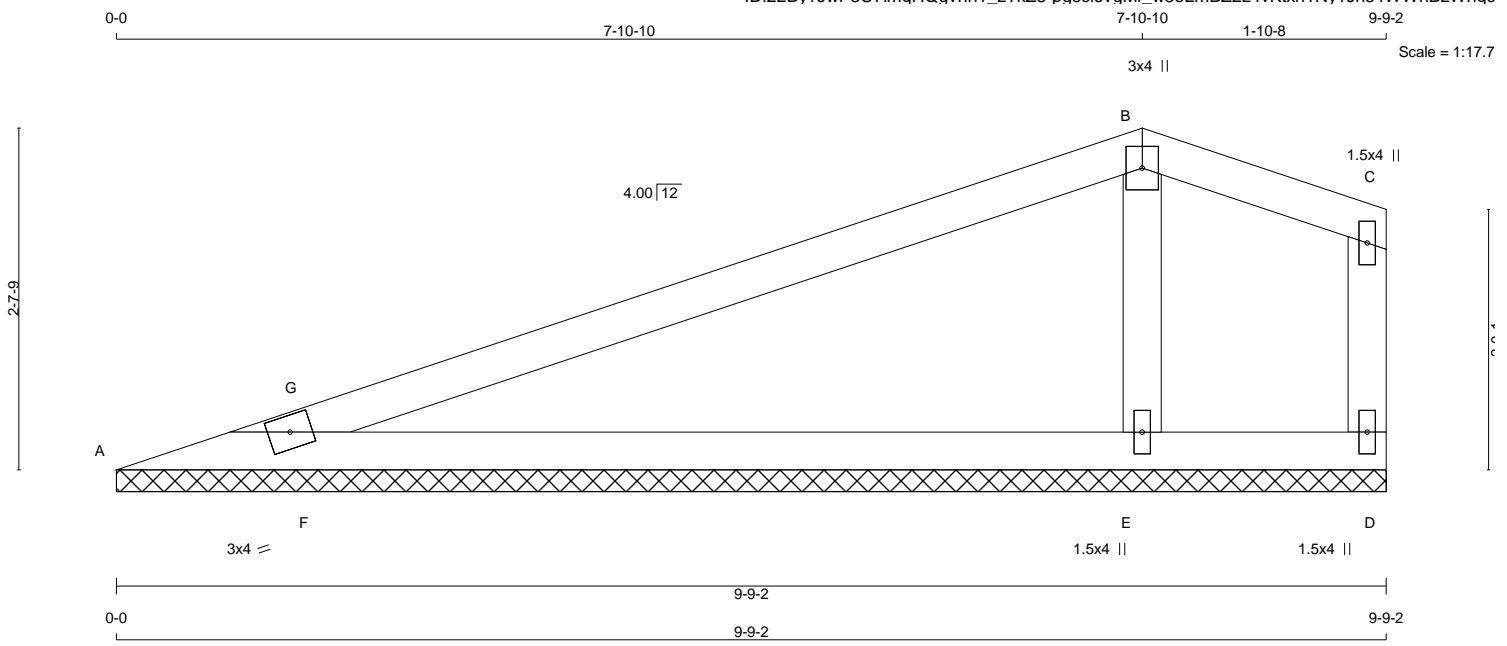
JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	2100F 1.8E	SPF
B - C	2x4 DRY	1650F 1.5E	SPF
D - C	2x4 DRY	1650F 1.5E	SPF
A - D	2x4 DRY	1650F 1.5E	SPF
ALL WEBS	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	3.0	4.0		
C	TMV+p	MT20	1.5	4.0		
D	BMV1+p	MT20	1.5	4.0		
E	BMV1+w	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	565	0	565	0	9-9-2	2-8-14
D	-77	0	0	0	-77 9-9-2	2-8-14
E	1042	0	1042	0	9-9-2	2-8-14

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	391	308 / 0	0 / 0	0 / 0	0 / 0	83 / 0
D	-55	0 / -29	0 / 0	0 / 0	0 / 0	0 / -26
E	724	547 / 0	0 / 0	0 / 0	0 / 0	177 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-G	-9 / 0	-139.4 -139.4	0.09 (4)	10.00	E-B	-580 / 0	0.04 (1)
G-B	0 / 3	-139.4 -139.4	0.65 (1)	10.00	F-G	-506 / 0	0.00 (1)
B-C	0 / 0	-139.4 -139.4	0.07 (1)	10.00			
D-C	-131 / 0	0.0	0.01 (1)	7.81			
A-F	-38 / 0	-17.5 -17.5	0.73 (1)	6.25			
F-E	0 / 0	-17.5 -17.5	0.73 (1)	10.00			
E-D	0 / 0	-17.5 -17.5	0.48 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.65/1.00 (B-G:1), BC=0.73/1.00 (A-F:1), WB=0.04/1.00 (B-E:1), SSI=0.33/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

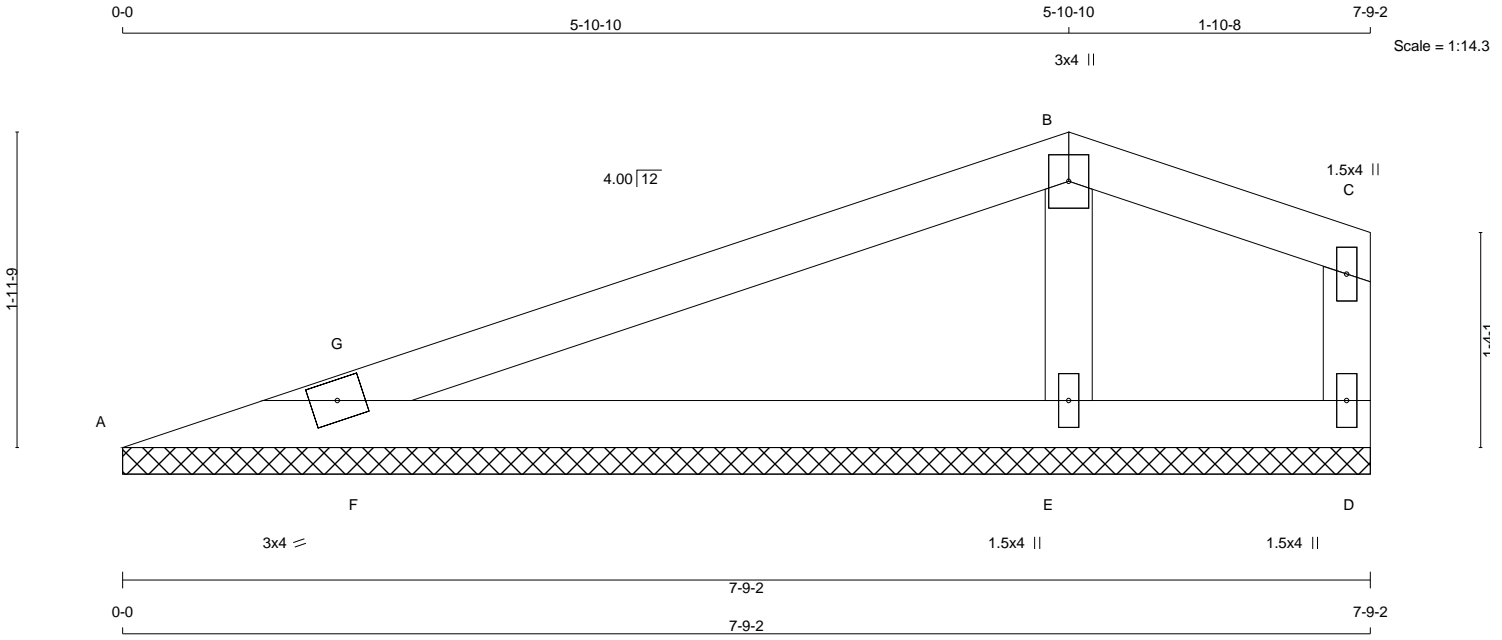
JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	1650F 1.5E	SPF
B - C	2x4 DRY	1650F 1.5E	SPF
D - C	2x4 DRY	1650F 1.5E	SPF
A - D	2x4 DRY	1650F 1.5E	SPF
ALL WEBS	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	3.0	4.0		
C	TMV+p	MT20	1.5	4.0		
D	BMV1+p	MT20	1.5	4.0		
E	BMV1+w	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	419	0	419	0	7-9-2	1-7-12
D	12	0	12	0	-6	7-9-2
E	787	0	787	0	0	7-9-2

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	289	227 / 0	0 / 0	0 / 0	0 / 0	62 / 0
D	7	12 / 0	0 / 0	0 / 0	0 / 0	0 / -5
E	546	417 / 0	0 / 0	0 / 0	0 / 0	129 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO	
A-G	-10 / 0	-139.4 -139.4	0.10 (1)	10.00	E-B	-447 / 0
G-B	0 / 5	-139.4 -139.4	0.41 (1)	10.00	F-G	-351 / 0
B-C	0 / 0	-139.4 -139.4	0.07 (1)	10.00		
D-C	-131 / 0	0.0	0.01 (1)	7.81		
A-F	-23 / 0	-17.5 -17.5	0.45 (1)	6.25		
F-E	0 / 0	-17.5 -17.5	0.45 (1)	10.00		
E-D	0 / 0	-17.5 -17.5	0.29 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (B-G:1), BC=0.45/1.00 (A-F:1), WB=0.03/1.00 (B-E:1), SSI=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

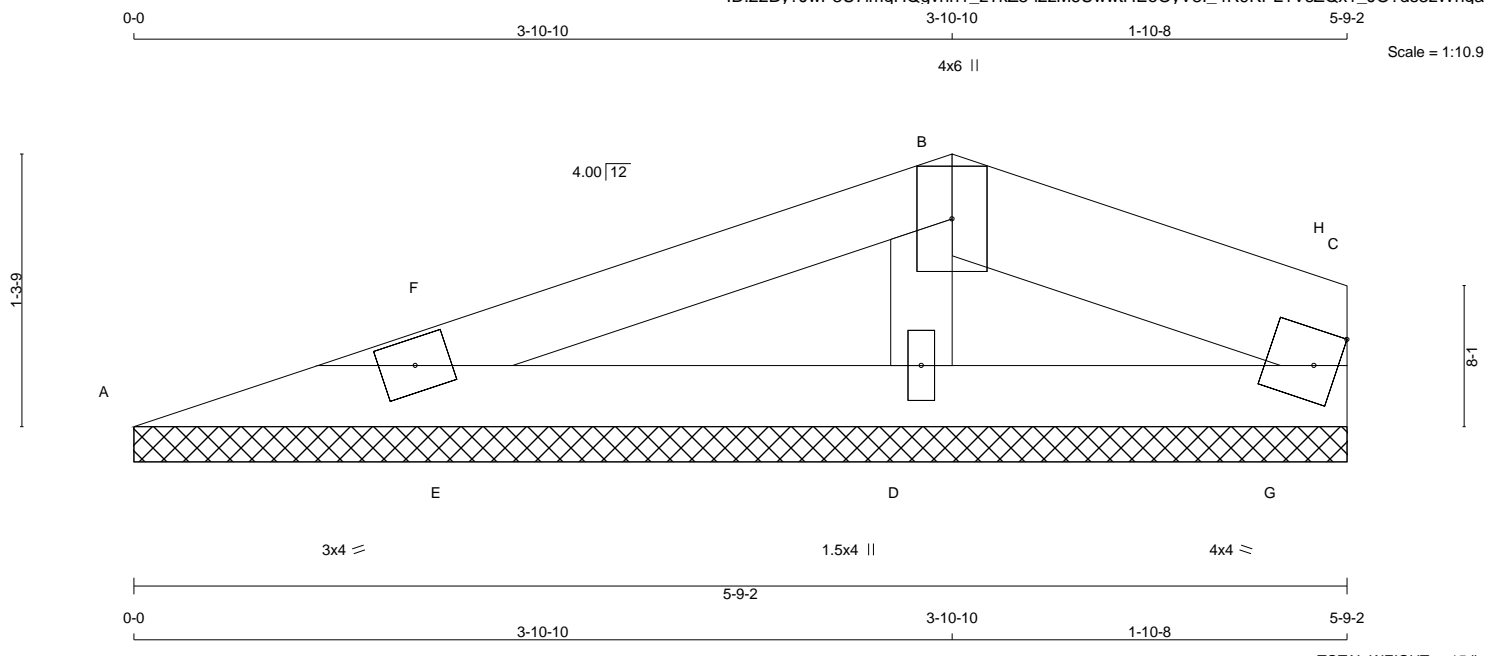
JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MITek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	1650F 1.5E	SPF
B - C	2x6 DRY	1650F 1.5E	SPF
A - C	2x4 DRY	1650F 1.5E	SPF
ALL WEBS	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	4.0	6.0		
C	TBM1-h	MT20	4.0	4.0		Edge
D	BMW1+w	MT20	1.5	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	206	0	206	0	5-9-2	1-1-1
C	-4	0	0	0	5-9-2	1-1-1
D	702	0	702	0	5-9-2	1-1-1

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	142	111 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	-3	0 / -1	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
D	486	377 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO
A-F	0 / 267	-139.4	-139.4	0.11 (1)	10.00	D-B	-461 / 0	0.03 (1)
F-B	0 / 284	-139.4	-139.4	0.16 (1)	10.00	E-F	-207 / 0	0.00 (1)
B-H	0 / 211	-139.4	-139.4	0.04 (1)	10.00	G-H	-289 / 0	0.00 (1)
H-C	0 / 306	-139.4	-139.4	0.03 (1)	10.00			
A-E	-276 / 0	-17.5	-17.5	0.17 (1)	6.25			
E-D	-266 / 0	-17.5	-17.5	0.17 (1)	6.25			
D-G	-196 / 0	-17.5	-17.5	0.13 (1)	6.25			
G-C	-196 / 0	-17.5	-17.5	0.06 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (B-F:1), BC=0.17/1.00 (D-E:1), WB=0.03/1.00 (B-D:1), SSI=0.17/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

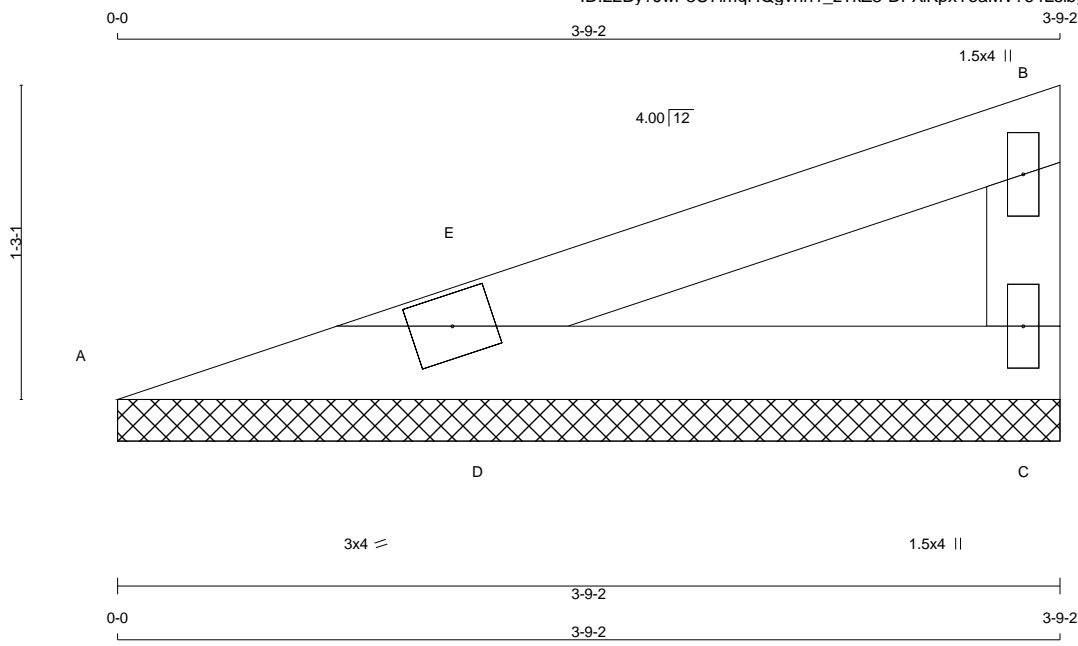
JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, alexandria, VA 22314.



TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	1650F 1.5E	SPF
C - B	2x4 DRY	1650F 1.5E	SPF
A - C	2x4 DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0	
B	TMV+p	MT20	1.5	4.0	
C	BMV1+p	MT20	1.5	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	295	0	295	0	3-9-2	3-9
C	295	0	295	0	3-9-2	3-9

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
A	204	159 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	204	159 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED CS (LC)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED CS (LC)	MEMB. LENGTH FR-TO
A-E	-15 / 0	-139.4	-139.4 0.12 (1)	6.25	D-E	-151 / 6 0.00 (1)
E-B	0 / 15	-139.4	-139.4 0.16 (1)	10.00		
C-B	-199 / 0	0.0	0.0 0.01 (1)	7.81		
A-D	-10 / 0	-17.5	-17.5 0.19 (1)	10.00		
D-C	0 / 0	-17.5	-17.5 0.19 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (B-E:1), BC=0.19/1.00 (A-D:1), WB=0.00/1.00 (D-E:1), SSI=0.15/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

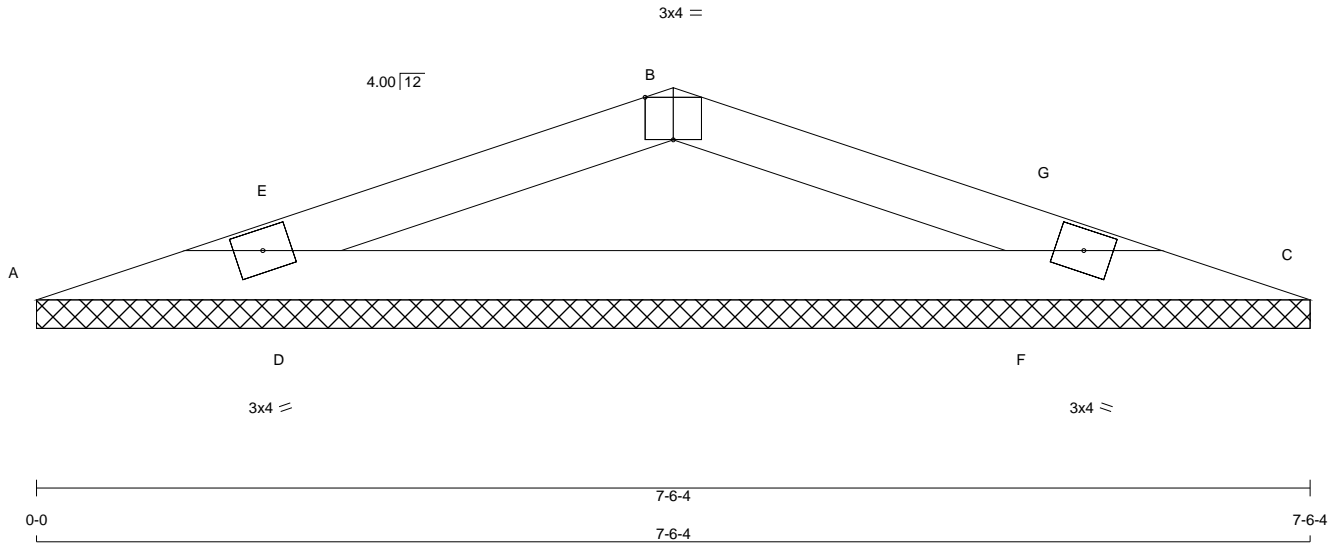
JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design valid for use only with MiTek connectors. This design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.



TOTAL WEIGHT = 16 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	1650F 1.5E	SPF
B - C	2x4	1650F 1.5E	SPF
A - C	2x4	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0	
B	TT-p	MT20	3.0	4.0	Edge 2.00
C	TBM1-h	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	590	0	590	0	7-6-4	4-8
C	590	0	590	0	7-6-4	4-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS						
		1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
A	408	318 / 0	0 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	408	318 / 0	0 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX FACTORED FORCE (LBS)	MAX CSI (LC)	MAX UNBRAC LENGTH FR-TO
A-E	-1034 / 0	-139.4	-139.4	0.42 (1)	6.13	D-E	0 / 65	0.00 (1)
E-B	-954 / 0	-139.4	-139.4	0.42 (1)	6.25	F-G	0 / 65	0.00 (1)
B-G	-954 / 0	-139.4	-139.4	0.42 (1)	6.25			
G-C	-1034 / 0	-139.4	-139.4	0.42 (1)	6.13			
A-D	0 / 950	-17.5	-17.5	0.29 (1)	10.00			
D-F	0 / 955	-17.5	-17.5	0.34 (1)	10.00			
F-C	0 / 950	-17.5	-17.5	0.29 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 42.3 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 54.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 73.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) EQUALS 42.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (B-E:1), BC=0.34/1.00 (D-F:1), WB=0.00/1.00 (D-E:1), SSI=0.26/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (A) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)



This document has been electronically certified with digital certificate and encryption technology authorized by APEGBC. The authoritative original is in electronic form transmitted to you. Any printed version can be relied upon as a true copy of the original when supplied by the original author, bearing images of the professional seal and digital certificate or when printed from the digitally certified electronic file sent to you.

03/29/2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473C rev 10-08 BEFORE USE
Design is based upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Bracing shown is for lateral support of individual web members only. additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component safety Information available from Truss Plate Institute, 781-N. Lee Street, Alexandria, VA 22314.